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THE DETERMINATION OF DEFLECTION AND STRESS DISTRIBUTION FOR A T--ETC(U)

NOV 76 P H DENKE, J B HOFFMAN

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THE DETERMINATION OF DEFLECTION AND STRESS  
DISTRIBUTION FOR A TRANSPARENT LAMINATED BEAM

Douglas Aircraft Company  
McDonnell Douglas Corporation  
3855 Lakewood Boulevard  
Long Beach, California 90846



MARCH 1977

Final Report for Period January 1976-March 1977

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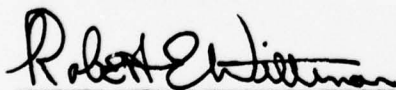
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20. ABSTRACT (Continue on reverse side if necessary and identify by block number)  This report documents the theory and procedures for calculating the elastic deflection and stress distribution of a flat, laminated beam with fixed ends or simply-supported ends comprised of transparency materials subjected to a normal point load at the midpoint of the beam. The development of a computer program using the basic theory is presented along with illustrative examples and a "user's manual." The program results have been favorably compared with actual test data and with a finite element method.		

## FOREWORD

This report is one of a series of reports that describes work performed by Douglas Aircraft Company, McDonnell Douglas Corporation, 3855 Lakewood Blvd., Long Beach, California 90846, under the Windshield Technology Demonstrator Program. This work was sponsored by the U.S. Air Force Flight Dynamics Laboratory, Wright-Patterson Air Force Base, under Contract F33615-75-C-3105, Project 2202/0201.

Captain D. C. Chapin (AFFDL/FEW) was the Air Force Project Manager who monitored the program and provided reference material on a timely basis.

Mr. J. H. Lawrence, Jr. was Program Director for the Douglas Aircraft Company.

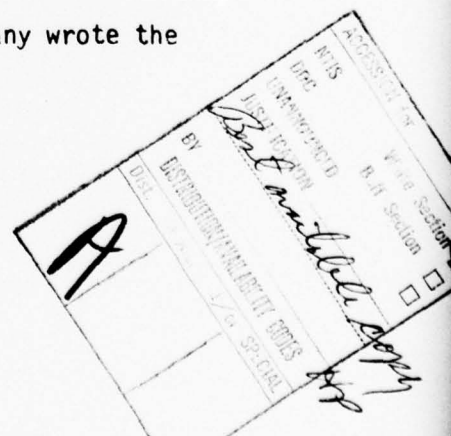
Mr. P. H. Denke and Mr. J. B. Hoffman were the principal investigators and co-authors for this report.

Mr. P. H. Denke of Douglas Aircraft Company developed and solved the differential equations presented in this report.

Mr. J. F. Burke and Mr. R. C. Morris of Douglas Aircraft Company and Mr. L. Chahinian of the McDonnell Douglas Automation Company provided valuable assistance in the preparation of the Computer Program User's Manual in this report.

Mr. J. C. Thomsen of Douglas Aircraft Company developed the finite element model presented in this report.

Mr. T. Price of McDonnell Douglas Automation Company wrote the computer program described in this report.



Dr. P. Poirier (ASD/ADDs) and Dr. P. J. Nikolai (AFFDL/FB), Wright-Patterson Air Force Base, Dayton, Ohio, provided the EISPACK sub-routines utilized in the Source Coding, Appendix B, in this report.

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# LIST OF ABBREVIATIONS AND SYMBOLS

(Used in Analysis, Section II)

$A$	A square coefficient matrix in the governing matrix differential equation
$A_i$	Cross-sectional area of $i^{\text{th}}$ layer
$a_{1,2}$	$k_A^{-1}$
$a_{2,1}$	$\Delta q \ k_{\eta} \ \Delta q^T$
$a_{2,3}$	$-\Delta q \ k_{\eta} \ \bar{t}$
$a_{5,4}$	$K_I^{-1} \bar{t}^T k_{\eta} \bar{t}$
$a_{5,2}$	$K_I^{-1} a_{2,3}^T a_{1,2}$
$B$	Coefficient matrix in the boundary condition equation
$b$	Beam width
$C$	A column matrix of arbitrary constants
$\bar{C}$	Column matrix of unit elements
$C_{A_k}$	Coefficient of $k^{\text{th}}$ algebraic mode
$C_{g_k}$	$\bar{C}_{g_k} e^{\lambda g k^{\ell}}$

$C_k$	$k^{\text{th}}$ arbitrary constant
$C_\ell, C_g, C_A$	Column matrices of the arbitrary constants $C_{\ell_k}, C_{g_k}$ , and $C_{A_k}$
$C_{\ell_k}, \bar{C}_{g_k}$	Arbitrary constants in the expressions for $Y_{E\ell}$ and $Y_{Eg}$
$e$	Base of natural logarithms
$E_i$	Young's modulus of $i^{\text{th}}$ layer
$EI_{\text{eff}}$	Effective beam bending stiffness
$F$	Load at beam centerline
$F_{g_k}(x)$	$e^{\lambda_{\ell_k}(\ell-x)}$
$F_\ell(x), F_g(x)$	Column matrices of the $F_{\ell_k}(x)$ 's and the $F_{g_k}(x)$ 's
$F_{\ell_k}(x)$	$e^{\lambda_{\ell_k}x}$
$F_{\ell D}(x), F_{gD}(x)$	$F_\ell(x), F_g(x)$ diagonalized
$G$	An eigenvector of $A$
$G_{g_k}$	$k^{\text{th}}$ eigenvector of $A$ corresponding to $\lambda_{g_k}$



$G_i$	Shear modulus of the $i^{\text{th}}$ layer
$G_k$	$k^{\text{th}}$ eigenvector of A
$G_{\ell k}$	$k^{\text{th}}$ eigenvector of A corresponding to $\lambda_{\ell k}$
$\left. \begin{array}{l} G_{u_k}, G_{H_k} \\ G_{v_k}, G_{\theta_k} \\ G_{\beta_k}, G_{\phi_k} \end{array} \right\}$	Partitions of $G_k$ corresponding to the partitions $u, H, v, \theta, \beta,$ and $\phi$ of Y
H	Column matrix of structural ply axial loads
$H_i$	Axial load in $i^{\text{th}}$ layer
$H_{\ell}, H_g$	Rectangular matrices consisting of the modal columns $G_{\ell k}$ and $G_{gk}$
I	Unit (identity) matrix
$I_i$	Cross-sectional moment of inertia of $i^{\text{th}}$ layer
$k_A$	Diagonal matrix of structural ply axial stiffnesses
$K_I$	Sum of cross-sectional moments of inertia of structural plies
$k_I$	Column matrix of structural ply bending stiffnesses
$k_{\ell}$	Column matrix of interlayer shear stiffnesses
$k_{\eta}$	Diagonal matrix of interlayer shear stiffnesses

$L$	Length of beam
$\ell$	$L/2$
$M$	Column matrix of structural ply bending moments
$M_i$	Bending moment in $i^{\text{th}}$ layer
$M_{xq}$	A matrix of structural ply thicknesses
$O$	Null matrix
$P$	$F/2$
$Q$	A Boolean integrating matrix
$q$	Column matrix of interlayer shear flows
$q_i$	Shear flow on lower surface of $i^{\text{th}}$ layer
$S$	$a_{2,1}$
$S(x)$	Total shear on the beam cross section
$\left. \begin{array}{l} S_{1,1}, S_{1,2} \\ S_{2,1}, S_{2,2} \end{array} \right\}$	Partitions of $S$
$T$	(Superscript) Indicates transposed matrix
$T$	$k_A^u(0)$
$\bar{t}$	$t_\ell + \bar{t}_s$

$t_i$	Thickness of $i^{\text{th}}$ layer
$t_\ell$	Column matrix of interlayer thicknesses
$t_s$	Column matrix of structural ply thicknesses
$\bar{t}_s$	Column matrix of average structural ply thicknesses
$T_1, T_2$	Partitions of $T$
$u$	Column matrix of longitudinal displacements at layer centerlines
$u_i$	Longitudinal displacement at centerline of $i^{\text{th}}$ layer
$u_{(0)}$	A column matrix of longitudinal displacements of structural ply centerlines in the rigid body translation parallel to $x$ algebraic mode
$u_{(1)}$	A column matrix of longitudinal displacements of structural ply centerlines in the rigid body rotation about $y$ algebraic mode
$u_{(2)}$	A column matrix of additional longitudinal displacements of structural ply centerlines in the shear parallel to $z$ algebraic mode
$\bar{u}_{(2)}$	A partition of $u_{(2)}$
$v$	Column matrix of structural ply transverse shears
$v_i$	Shear in $i^{\text{th}}$ layer

$v_i, v$	Vertical displacement at centerline of $i^{\text{th}}$ layer
$W$	$k_A Q \bar{t}$
$w_i$	Normal load per unit length on lower surface of $i^{\text{th}}$ layer
$W_1, W_2$	Partitions of $W$
$x, y, z$	Coordinates with origin at beam centerline
$Y_A(x)$	Rectangular matrix of the algebraic modes
$Y_{A_k}(x)$	$k^{\text{th}}$ algebraic mode
$\left. \begin{array}{l} Y_{Au}(x), Y_{AH}(x) \\ Y_{Av}(x), Y_{A\theta}(x) \\ Y_{A\beta}(x), Y_{A\phi}(x) \end{array} \right\}$	Partitions of $Y_A(x)$ corresponding to the partitions $u, H, v, \theta, \beta$ , and $\phi$ of $Y$
$Y_E$	A column matrix representing the exponential beam response
$Y_{E_\ell}$	A column matrix representing exponential beam responses corresponding to eigenvalues $\lambda_{\ell k}$
$Y_{E_g}$	A column matrix representing exponential beam responses corresponding to eigenvalues $\lambda_{gk}$
$Y(x)$	A column matrix of beam responses

$\alpha$	A scalar coefficient of $u_{(0)}$ in the equation for $u_{(1)}$
$\beta_i, \beta$	Curvature of $i^{\text{th}}$ layer = $d\theta_i/dx$
$\gamma_i$	Shear strain of $i^{\text{th}}$ layer
$\Delta_\eta$	A longitudinal displacement differencing matrix
$\Delta q$	A shear flow differencing matrix
$\Delta_{ts}$	A Boolean coefficient matrix
$\bar{\Delta}_\eta$	Column matrix of longitudinal displacement differences at layer interfaces
$\epsilon_{x_i}$	Axial strain of $i^{\text{th}}$ layer
$\zeta_i$	Vertical displacement of lower surface of $i^{\text{th}}$ layer
$\eta$	Column matrix of longitudinal displacements at layer interfaces
$\eta_i$	Longitudinal displacement of lower surface of $i^{\text{th}}$ layer
$\theta_i, \theta$	Slope of $i^{\text{th}}$ layer = $dv_i/dx$
$\lambda$	An eigenvalue of A
$\lambda_{g_k}$	$k^{\text{th}}$ eigenvalue of A greater than zero



$\lambda_k$	$k^{\text{th}}$ eigenvalue of A
$\lambda_{\ell k}$	$k^{\text{th}}$ eigenvalue of A less than zero
$\Sigma$	Summation sign
$\Sigma_u$	A Boolean coefficient matrix
$\phi$	$dB/dx$
$\bar{\phi}_i$	Rotation of a vertical section of the $i^{\text{th}}$ layer
$\psi$	Column matrix of constants in the boundary condition equation

## SECTION I

### INTRODUCTION

Bird impact hazards to high speed, low flying, aircraft have become one of the major flight safety problems of the jet age. The Douglas Aircraft Company's Windshield Technology Demonstrator Program is part of an effort by the Air Force Flight Dynamics Laboratory Improved Windshield Protection Program to develop technologies that will allow the design of aircraft transparent enclosures to increase protection against birdstrikes.

One of the basic objectives of the Windshield Technology Demonstrator Program is to develop windshield design technology for application to high performance military aircraft. Pursuant to this effort, a need arose for a simplified analytical method of rapidly assessing the structural effectiveness of candidate laminate configurations.

Typical laminated configurations consist of alternating structural plies and interlayers. Structural plies are composed of high modulus materials such as polycarbonate, glass or acrylic. Interlayers are composed of materials that may be less stiff by several orders of magnitude.

A realistic method of static analysis for a laminated beam carrying a concentrated load at the center, representing a bird impact, and having various types of end supports, would be a useful tool. Such a method should avoid the frequently made Bernoulli-Euler assumption that plane sections remain plain during deformation, because shear deformations in the soft interlayers are large and must be considered. The significance of these deformations is shown experimentally in Reference 1. Other rough assumptions that would unnecessarily diminish the effectiveness of the analysis should also be avoided. The method should accommodate laminates composed of nine layers or more, with different material properties for each layer.

Apparently no method meeting these requirements exists in the literature, although related work is described. Reference 2 contains an analysis of a three layered beam based on the assumption that the angle of rotation between a cross section of the center layer and its undeformed position is a constant factor times the slope along the length of the beam. This assumption is considered unnecessary, and its effects on the reliability of the results are hard to assess. The solution is based on the energy method, which introduces further approximations. Reference 3 presents methods of analysis of multiple layered beams based on the Bernoulli-Euler hypothesis; consequently, the approach is not applicable to beams with soft interlayers. Reference 4 offers an analysis of three layered beams involving simplifying assumptions which are also not considered acceptable for the present application.

Therefore, a new approach to the problem was developed, and is presented in subsequent sections of this report. The approach is based on the assumption that each structural ply can be treated as a beam to which the Bernoulli-Euler hypothesis is applicable, but no such assumption is applied to the cross section as a whole. Structural plies can bend and stretch, but shear deformations of these plies are considered negligible. Interlayers are assumed to carry shear, but not axial loads or bending moments. Deformations through the thickness of the beam are considered negligible, but stresses normal to the layers are assumed to exist. The equations of equilibrium and compatibility are written. The resulting set of differential equations is then expressed as a single matrix differential equation, which is solved exactly.

The analytical results have been translated into an efficient Fortran program. This program applies to nine layer laminates, which is an adequate number in most cases. The program can be easily extended to cover more layers, if necessary. Fewer layers can be accommodated by introducing negligible stiffness properties ( $E$  and  $G$ ) for some of the layers. The method applies to fixed ended beams, but the program can be modified to be applicable to other boundary conditions. Extension of the method to other loading conditions is possible.

The most significant simplification involved in the analysis is the assumption that deformations through the thickness are negligible. This assumption greatly simplifies the analysis without slighting the primary feature of windshield laminate behavior, which is the relative freedom of structural plies to slide past each other because of the low stiffness of interlayer materials. The only negative effect of the assumption is that stresses normal to the layers are not correctly predicted. This is believed to be a localized effect confined to the center and ends of the beam where loads are applied. The elimination of this assumption is a possible subject for additional research. An analysis which accounts for transverse deformations might provide data that would be useful in defining adhesive strength needed to prevent delamination in regions of high transverse loads.

Results of the analysis have been correlated with finite element results and test data as described in a subsequent section. The comparisons are good and verify the validity of the basic assumptions.

Although the present method is intended to apply to fixed ended beams, it can also be applied to a beam with pinned ends by considering a fixed ended beam twice as long as the beam under consideration. The data output by the computer program between the quarter points of the fixed ended beam is applicable to the pin ended case.

## SECTION II

### ANALYSIS

#### DERIVATION OF EQUATIONS

This analysis applies to the fixed ended laminated beam of Figure 1. As the figure shows,  $L$  is the length of the beam, and  $F$  is the concentrated load acting at the center.

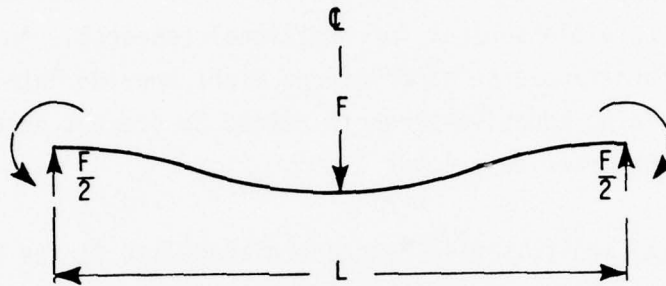


Figure 1. Fixed ended laminated beam.

The cross section is shown in Figure 2. The cross hatched layers are called "structural plies", while the other layers are "interlayers". The width of the beam is  $b$ , and the thickness of the  $i$ th layer is  $t_i$ . Different material properties can be assigned to each layer. The flexibilities of the interlayers are assumed to be large compared to the structural plies. The analysis can be applied to a beam having fewer than nine layers by assigning negligible values of Young's modulus,  $E$ , and the shear modulus,  $G$ , to some of the layers.



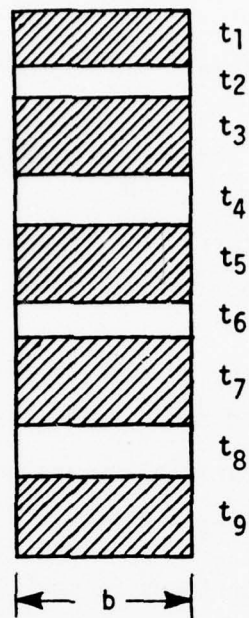


Figure 2. A typical transparency nine ply cross section.

Because of symmetry, consider one-half the beam as shown in Figure 3. In the figure  $\ell = L/2$ , and  $P = F/2$ . The figure also shows the xyz reference frame, and the definition of the vertical displacement  $v$ .

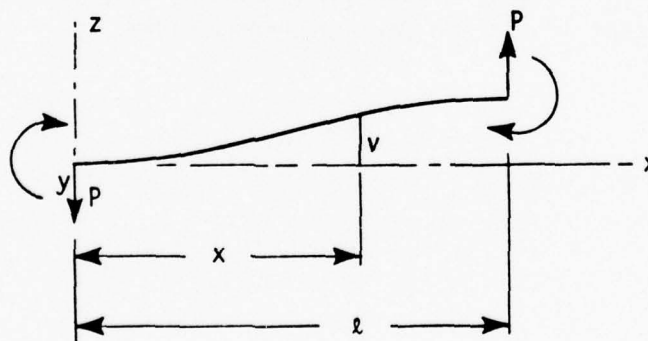


Figure 3. Half-beam.

### Assumptions

1. Structural plies carry axial load, bending moment, shear, and normal load.
2. Interlayers carry shear and normal stresses only.
3. Plane sections of structural plies remain plane and normal to their elastic axes during deformation.
4. Normal strains through the thickness for structural plies and interlayers are negligible and can be considered equal to zero.
5. Shear strains for structural plies are negligible.
6. All stress-strain relations are linear.
7. Structural ply bending conforms to small displacement theory.

### Equilibrium, Structural Ply Element, $i$ Odd

Figure 4 shows the equilibrium of an element of the  $i$ th structural ply.  $H_i$ ,  $V_i$  and  $M_i$  are the axial load, shear and bending moment acting on the element cross section. The transverse load per inch,  $w_i$ , and the shear flow,  $q_i$ , act upon the lower face. The corresponding forces on the upper face are  $w_{i-1}$  and  $q_{i-1}$ , since the  $i-1$ st element is next above. Note that  $i$  is odd for structural plies.

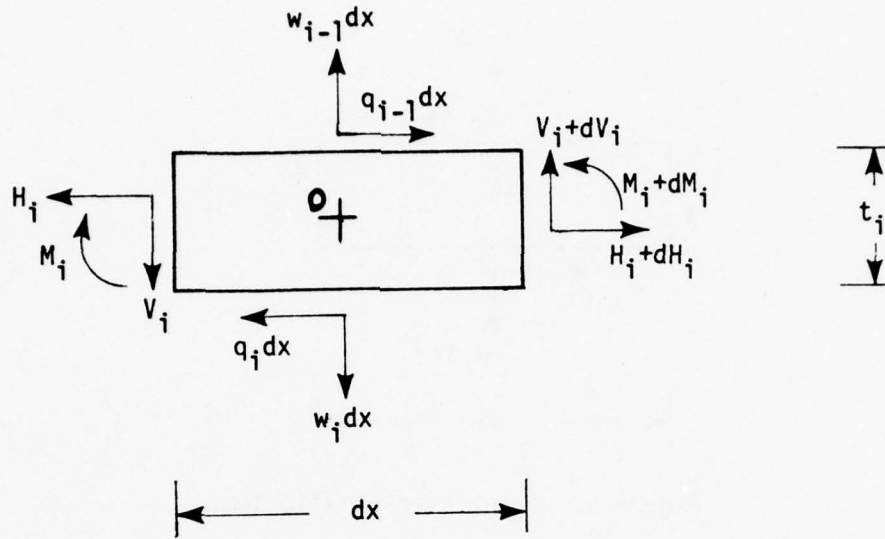


Figure 4. Structural ply equilibrium.

Summing forces in two directions and moments about 0 gives:

$$\left\{ \begin{array}{ll} \frac{dH_i}{dx} + q_{i-1} - q_i = 0 & (i \text{ odd}) \quad (1) \\ \frac{dV_i}{dx} + w_{i-1} - w_i = 0 & (i \text{ odd}) \quad (2) \\ \frac{dM_i}{dx} + V_i - q_{i-1} \frac{t_i}{2} - q_i \frac{t_i}{2} = 0 & (i \text{ odd}) \quad (3) \end{array} \right.$$

where  $1/2 dV_i$  has been deleted from Equation 3 to reduce complexity of problem. This is allowable due to insignificance of the term.

#### Equilibrium, Interlayer Element, i Even

Figure 5 shows the equilibrium of an element of the  $i$ th interlayer. The notation is consistent with the notation established for structural plies, although no axial loads or bending moments are acting, because of the assumption that these forces are negligible for interlayers. Note that  $i$  is even for these plies.

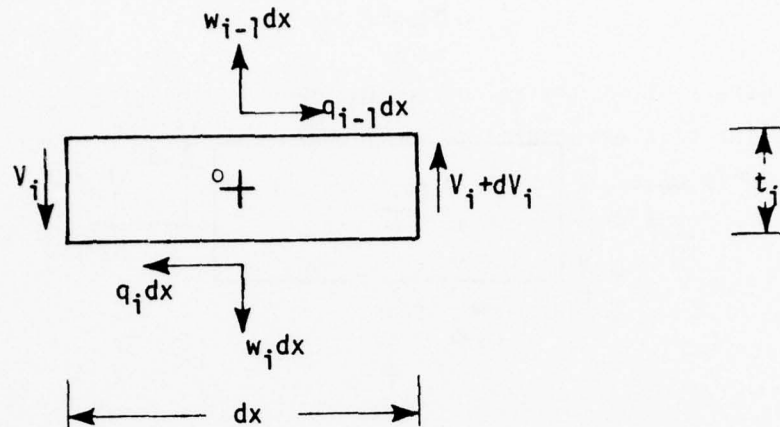


Figure 5. Interlayer equilibrium.

Summing forces in two directions and moments about 0 gives:

$$\begin{cases} q_{i-1} - q_i = 0 & (i \text{ even}) \end{cases} \quad (4)$$

$$\begin{cases} \frac{dV_i}{dx} + w_{i-1} - w_i = 0 & (i \text{ even}) \end{cases} \quad (5)$$

$$\begin{cases} V_i - q_{i-1} \frac{t_i}{2} - q_i \frac{t_i}{2} = 0 & (i \text{ even}) \end{cases} \quad (6)$$

From Equation 4,  $q_{i-1} = q_i$ , and, from Equation 6,  $V_i = q_i t_i$ .

Therefore, from Equation 5,

$$t_i \frac{dq_i}{dx} + w_{i-1} - w_i = 0 \quad (i \text{ even}) . \quad (6a)$$

Also note that

$$q_0 = q_9 = 0 \quad (6b)$$

$$w_0 = w_9 = 0 \quad (6c)$$

since transverse forces and shear flows are assumed to be zero on the upper and lower surfaces of the beam.

### Compatibility, Structural Plies, $i$ Odd

Figure 6 shows the relationships between the displacements of the centerline of a structural ply and the displacements of the upper and lower surfaces of the same layer. In the figure,  $u_i$ ,  $v_i$  and  $\theta_i$  are the longitudinal and transverse displacements and the slope of the centerline of the  $i$ th ply. The longitudinal and transverse displacements of the lower surface of the  $i$ th ply are denoted  $\eta_i$  and  $\zeta_i$ . Consequently the corresponding displacements of the upper surface are  $\eta_{i-1}$  and  $\zeta_{i-1}$ , since the upper surface of the  $i$ th ply is the lower surface of the  $(i-1)$ st ply.

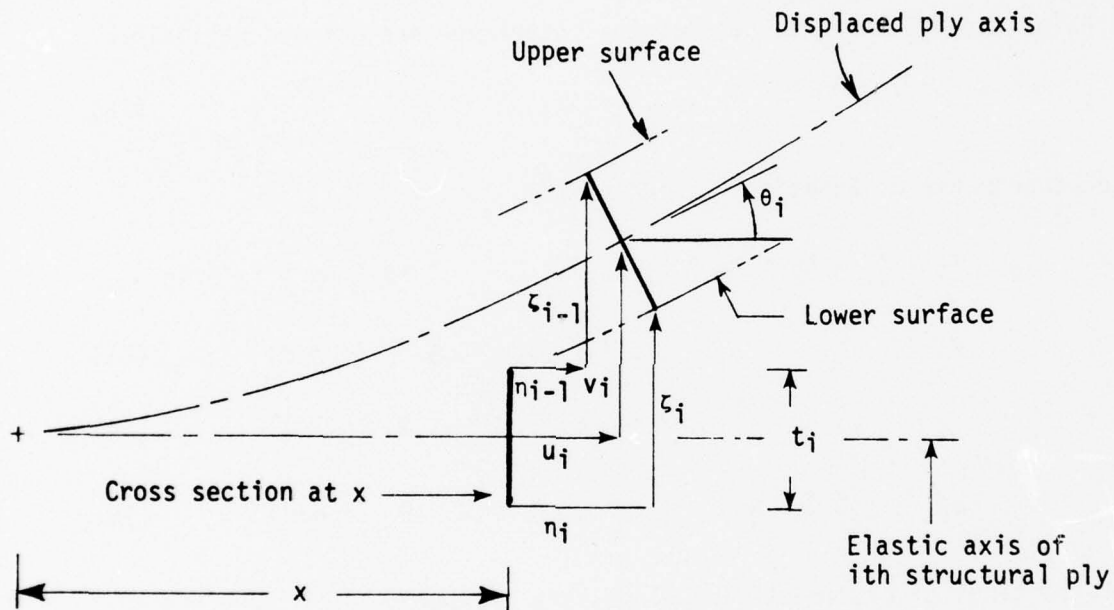


Figure 6. Compatibility.

$$\text{Then from Figure 6, } \eta_{i-1} = u_i - \theta_i \frac{t_i}{2} \quad (7)$$

$$\text{and } \eta_i = u_i + \theta_i \frac{t_i}{2} \text{ for small displacements. } (8)$$

These equations are consistent with the assumption that cross sections of the structural ply remain normal to the elastic axis during deformation.



Also from Figure 6,

$$\zeta_{i-1} = v_i, \quad (9)$$

$$\zeta_i = v_i, \quad (10)$$

$$\text{and} \quad \theta_i = \frac{dv_i}{dx}. \quad (11)$$

Equations 9 and 10 hold because strains through the thickness of structural plies are considered negligible. Equation 11 is consistent with the assumption that structural ply bending conforms to the theory of small displacements.

#### Compatibility, Interlayers, i Even.

Strains through the thickness of the interlayer are assumed negligible.

$$\therefore \quad \zeta_{i-1} = \zeta_i. \quad (12)$$

In consequence of Equations 9, 10 and 12:

$$\zeta_0 = v_1 = \zeta_1 = \zeta_2 = v_3 = \zeta_3 = \zeta_4 = \dots = \zeta_7 = \zeta_8 = v_9 = \zeta_9,$$

$$\therefore \quad v_i = v \quad i = 1, 3, \dots, 9 \quad (i \text{ odd}) \quad (13)$$

$$\zeta_i = v \quad i = 0, 1, 2, \dots, 9 \quad (\text{all } i) \quad (14)$$

$$\theta_i = \frac{dv}{dx} = \theta \quad i = 1, 3, \dots, 9 \quad (i \text{ odd}). \quad (15)$$

All vertical displacements and slopes are the same.

#### Strains, Structural Plies, i Odd.

Longitudinal strain,  $\epsilon$ , is equal to the derivative of longitudinal displacement:

$$\epsilon_{x_i} = \frac{du_i}{dx} \quad \text{axial strain.}$$

In small displacement theory, curvature  $\beta$  is equal to the rate of change of slope, therefore,

$$\beta_i = \frac{d\theta_i}{dx} = \frac{d\theta}{dx} = \beta . \quad (15a)$$

The notation,  $\frac{d\beta}{dx} = \phi$  , (15b)  
to denote the rate of change of beam curvature, is useful subsequently.

#### Strains, Interlayers, i Even

Figure 7 shows the relationships between the shear strain of the  $i$ th interlayer,  $\gamma_i$  , the slope of the beam,  $\theta$  , and the longitudinal displacement of the  $i$ th interlayer upper and lower surfaces. In the figure,  $\bar{\phi}_i$  denotes the rotation of a cross section of the  $i$ th interlayer relative to its undisplaced position.

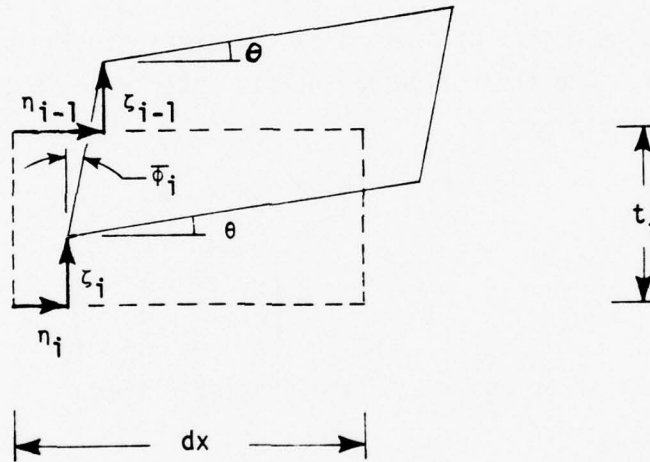


Figure 7. Interlayer shear strain.

From Figure 7,  $\gamma_i = \theta + \bar{\phi}_i$  (15c)

$\therefore \gamma_i = \frac{dv}{dx} + \frac{\eta_{i-1} - \eta_i}{t_i} ,$  (15d)

### Force-Deformation Relations

The longitudinal force on the  $i$ th interlayer is given in terms of the longitudinal strain by

$$H_i = E_i A_i \epsilon_{x_i} .$$

$$\therefore H_i = E_i A_i \frac{du_i}{dx} \quad (i \text{ odd}) \quad (16)$$

where  $E_i$  and  $A_i$  are the values of Young's modulus and the cross-sectional area of the  $i$ th structural ply. The bending moment in the  $i$ th structural ply is given in terms of the curvature by

$$M_i = E_i I_i \beta_i .$$

$$\therefore M_i = E_i I_i \frac{d^2v}{dx^2} \quad (i \text{ odd}) \quad (17)$$

where  $I_i$  is the moment of inertia of the cross section of the  $i$ th structural ply. The shear flow in the  $i$ th interlayer is given in terms of the shear strain by

$$q_i = G_i b \gamma_i .$$

$$\therefore q_i = G_i b \left( \frac{dv}{dx} + \frac{\eta_{i-1} - \eta_i}{t_i} \right) \quad (i \text{ even}) \quad (18)$$

where  $G_i$  is the shear modulus of the  $i$ th interlayer.

### Matric Formulation

Equation 1 can be written for each structural ply thus:

$$\frac{dH_1}{dx} + q_0 - q_1 = 0$$

$$\frac{dH_3}{dx} + q_2 - q_3 = 0$$

$$\frac{dH_5}{dx} + q_4 - q_5 = 0$$

$$\frac{dH_7}{dx} + q_6 - q_7 = 0$$

$$\frac{dH_9}{dx} + q_8 - q_9 = 0$$

But, from Equations 4 and 6b:

$$q_0=0 \quad q_1=q_2 \quad q_3=q_4 \quad q_5=q_6 \quad q_7=q_8 \quad q_9=0$$

$$\therefore \quad \frac{dH_1}{dx} - q_2 = 0 ,$$

$$\frac{dH_3}{dx} + q_2 - q_4 = 0 ,$$

$$\frac{dH_5}{dx} + q_4 - q_6 = 0 ,$$

$$\frac{dH_7}{dx} + q_6 - q_8 = 0 ,$$

$$\frac{dH_9}{dx} + q_8 = 0 .$$

Writing these equations in matrix form gives:

$$\begin{pmatrix} \frac{dH_1}{dx} \\ \frac{dH_3}{dx} \\ \frac{dH_5}{dx} \\ \frac{dH_7}{dx} \\ \frac{dH_9}{dx} \end{pmatrix} + \begin{bmatrix} -1 & & & & \\ & 1 & -1 & & \\ & & 1 & -1 & \\ & & & 1 & -1 \\ & & & & 1 \end{bmatrix} \begin{pmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \end{pmatrix} = 0 \quad (18a)$$

$\frac{dH}{dx}$

$\Delta q$

$q$

Equation 18a has been derived in detail to show the method of converting the scalar differential equations into matrix form. Subsequent matrix equations are derived in a similar manner, although the details are omitted.

The matrices in Equation 18a are denoted according to the symbols written under the equation. Thus,  $H$  is a column matrix of structural ply longitudinal forces,  $dH/dx$  is its derivative,  $q$  is a column matrix of interlayer shear flows, and  $\Delta_q$  is a Boolean differencing matrix.

Therefore,

$$\frac{dH}{dx} + \Delta_q q = 0. \quad (19)$$

From Equations 3, 4 and 6b,

$$\begin{pmatrix} \frac{dM_1}{dx} \\ \frac{dM_3}{dx} \\ \frac{dM_5}{dx} \\ \frac{dM_7}{dx} \\ \frac{dM_9}{dx} \end{pmatrix} + \begin{pmatrix} V_1 \\ V_3 \\ V_5 \\ V_7 \\ V_9 \end{pmatrix} - \begin{bmatrix} \frac{t_1}{2} & & & & \\ & \frac{t_3}{2} & & & \\ & & \frac{t_5}{2} & & \\ & & & \frac{t_7}{2} & \\ & & & & \frac{t_9}{2} \end{bmatrix} \begin{pmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \end{pmatrix} = 0.$$

$\frac{dM}{dx} \qquad V \qquad M_{xq} \qquad q$

The matrices are denoted according to the symbols written under the equation. The symbol  $M$  denotes a column matrix of structural ply bending moments,  $dM/dx$  is the derivative of  $M$ ,  $V$  is a column matrix of structural ply shear forces, and  $M_{xq}$  is a rectangular matrix of structural ply thicknesses.

$$\therefore \frac{dM}{dx} + V - M_{xq} q = 0. \quad (22)$$



From Equations 2, 6a and 6c:

$$\left\{ \begin{array}{l} \frac{dV_1}{dx} - w_1 = 0 \\ t_2 \frac{dq_2}{dx} + w_1 - w_2 = 0 \\ \frac{dV_3}{dx} + w_2 - w_3 = 0 \\ t_4 \frac{dq_4}{dx} + w_3 - w_4 = 0 \\ \frac{dV_5}{dx} + w_4 - w_5 = 0 \\ t_6 \frac{dq_6}{dx} + w_5 - w_6 = 0 \\ \frac{dV_7}{dx} + w_6 - w_7 = 0 \\ t_8 \frac{dq_8}{dx} + w_7 - w_8 = 0 \\ \frac{dV_9}{dx} + w_8 = 0 \end{array} \right.$$

Adding these equations gives

$$\frac{dV_1}{dx} + \frac{dV_3}{dx} + \frac{dV_5}{dx} + \frac{dV_7}{dx} + \frac{dV_9}{dx} + t_2 \frac{dq_2}{dx} + t_4 \frac{dq_4}{dx} + t_6 \frac{dq_6}{dx} + t_8 \frac{dq_8}{dx} = 0 . \quad (23)$$

The matrix form of this equation is:

$$\begin{bmatrix} 1 & 1 & 1 & 1 & 1 \end{bmatrix} \begin{Bmatrix} dV_1/dx \\ dV_3/dx \\ dV_5/dx \\ dV_7/dx \\ dV_9/dx \end{Bmatrix} + \begin{bmatrix} t_2 & t_4 & t_6 & t_8 \end{bmatrix} \begin{Bmatrix} dq_2/dx \\ dq_4/dx \\ dq_6/dx \\ dq_8/dx \end{Bmatrix} = 0$$

$$\bar{C}^T \quad \frac{dV}{dx} \quad t_\ell^T \quad \frac{dq}{dx}$$

The matrices are denoted according to the symbols written under the equation. The matrices  $dV/dx$  and  $dq/dx$  are the derivatives of the column matrices  $V$  and  $q$  previously defined,  $\bar{C}$  is a Boolean column matrix, and  $t_\ell$  is a column matrix of interlayer thicknesses. The superscript  $T$  indicates a transposed matrix.

$$\therefore \quad \bar{C}^T \frac{dV}{dx} + t_\ell^T \frac{dq}{dx} = 0. \quad (23a)$$

From Equation 16:

$$\begin{Bmatrix} H_1 \\ H_3 \\ H_5 \\ H_7 \\ H_9 \end{Bmatrix} = \begin{bmatrix} E_1 A_1 & & & & \\ & E_3 A_3 & & & \\ & & E_5 A_5 & & \\ & & & E_7 A_7 & \\ & & & & E_9 A_9 \end{bmatrix} \begin{Bmatrix} du_1/dx \\ du_3/dx \\ du_5/dx \\ du_7/dx \\ du_9/dx \end{Bmatrix}$$

$$H \quad k_A \quad \frac{du}{dx}$$

The matrices are denoted according to the symbols written under the equation. The symbol  $u$  denotes a column matrix of longitudinal displacements of structural ply centerlines,  $du/dx$  is the derivative of  $u$ , and  $k_A$  is a diagonal matrix of structural ply axial stiffnesses.

$$\therefore \quad H = k_A \frac{du}{dx} \quad (24)$$

From Equation 17:

$$\begin{Bmatrix} M_1 \\ M_3 \\ M_5 \\ M_7 \\ M_9 \end{Bmatrix} = \begin{Bmatrix} E_1 I_1 \\ E_3 I_3 \\ E_5 I_5 \\ E_7 I_7 \\ E_9 I_9 \end{Bmatrix} \frac{d^2 v}{dx^2}$$

$M \qquad k_I$

The matrices are denoted according to the symbols written under the equation. The symbol  $k_I$  denotes a column matrix of structural ply bending stiffnesses.

$$\therefore M = k_I \frac{d^2 v}{dx^2} \quad (25)$$

From Equation 18:

$$\begin{Bmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \end{Bmatrix} = \begin{Bmatrix} G_2 b \\ G_4 b \\ G_6 b \\ G_8 b \end{Bmatrix} \frac{dv}{dx} + \left[ \begin{array}{c|c|c|c} \frac{G_2 b}{t_2} & & & \\ & \frac{G_4 b}{t_4} & & \\ & & \frac{G_6 b}{t_6} & \\ & & & \frac{G_8 b}{t_8} \end{array} \right] \begin{Bmatrix} \eta_1 - \eta_2 \\ \eta_3 - \eta_4 \\ \eta_5 - \eta_6 \\ \eta_7 - \eta_8 \end{Bmatrix}$$

$q \qquad k_\ell \qquad k_n \qquad \overline{\Delta \eta}$

The matrices are denoted according to the symbols written under the equation. The symbol  $\overline{\Delta \eta}$  denotes a column matrix of longitudinal displacement differences at layer interfaces,  $k_\ell$  is a column matrix of interlayer shear stiffnesses, and  $k_n$  is a diagonal matrix of interlayer shear stiffnesses divided by interlayer thicknesses.

$$\therefore q = k_\ell \frac{dv}{dx} + k_n \overline{\Delta \eta}$$

But

$$\overline{\Delta \eta} = \Delta_n \eta$$

where

$$\Delta_n = \begin{bmatrix} 1 & -1 & & & \\ & 1 & -1 & & \\ & & 1 & -1 & \\ & & & 1 & -1 \end{bmatrix}$$

and

$$n = \{ n_1 | n_2 | n_3 | n_4 | n_5 | n_6 | n_7 | n_8 \} \quad (\text{column}) .$$

$\Delta n$  is a rectangular differencing matrix, and  $n$  is a column matrix of longitudinal displacements of layer interfaces.

$$\therefore q = k_\ell \frac{dv}{dx} + k_n \Delta n . \quad (26)$$

From Equations 7, 8 and 15,

$$\begin{pmatrix} n_1 \\ n_2 \\ n_3 \\ n_4 \\ n_5 \\ n_6 \\ n_7 \\ n_8 \end{pmatrix} = \begin{pmatrix} u_1 \\ u_3 \\ u_3 \\ u_5 \\ u_5 \\ u_7 \\ u_7 \\ u_9 \end{pmatrix} + \begin{pmatrix} t_1/2 \\ -t_3/2 \\ t_3/2 \\ -t_5/2 \\ t_5/2 \\ -t_7/2 \\ t_7/2 \\ -t_9/2 \end{pmatrix} \theta$$

$$\therefore \begin{pmatrix} n_1 \\ n_2 \\ . \\ . \\ . \\ n_8 \end{pmatrix} = \begin{bmatrix} 1 & & & & \\ & 1 & & & \\ & & 1 & & \\ & & & 1 & \\ & & & & 1 \end{bmatrix} \begin{pmatrix} u_1 \\ u_3 \\ u_3 \\ u_5 \\ u_5 \\ u_7 \\ u_7 \\ u_9 \end{pmatrix} + \frac{1}{2} \begin{bmatrix} 1 & & & & \\ & -1 & & & \\ & & -1 & & \\ & & & -1 & \\ & & & & -1 \end{bmatrix} \begin{pmatrix} t_1 \\ t_3 \\ t_5 \\ t_7 \\ t_9 \end{pmatrix} \theta$$

$n$ 
 $\Sigma_u$ 
 $u$ 
 $\Delta_{ts}$ 
 $t_s$

The matrices are denoted according to the symbols written under the equation. The symbol  $n$  denotes a column matrix of longitudinal displacements,  $t_s$  is a column matrix of structural ply thicknesses, and  $\Sigma_u$  and  $\Delta_{ts}$  are Boolean coefficient matrices.

$$\therefore n = \Sigma_u u + \frac{1}{2} \Delta_{ts} t_s \frac{dv}{dx} \quad (27)$$

From Equation 24,

$$\frac{du}{dx} - a_{1,2} H = 0 \quad (28)$$

where

$$a_{1,2} = k_A^{-1} \quad (29)$$

Eliminate  $n$  from Equations 26 and 27:

$$q = (k_\ell + k_n \cdot \frac{1}{2} \Delta_n \Delta_{ts} t_s) \frac{dv}{dx} + k_n \Delta_n \Sigma_u u$$

But  $k_\ell = k_n t_\ell$ ,  $\frac{1}{2} \Delta_n \Delta_{ts} t_s = \bar{t}_s$ , and  $\Delta_n \Sigma_u = -\Delta_q^T$ ,

where

$$\bar{t}_s = \begin{pmatrix} \frac{1}{2}(t_1 + t_3) \\ \frac{1}{2}(t_3 + t_5) \\ \frac{1}{2}(t_5 + t_7) \\ \frac{1}{2}(t_7 + t_9) \end{pmatrix}$$

and  $\bar{t}_s$  is a column matrix of average structural ply thicknesses.

$$\therefore q = k_n \bar{t} \frac{dv}{dx} - k_n \Delta_q^T u \quad (30)$$

where

$$\bar{t} = t_\ell + \bar{t}_s = \begin{pmatrix} \frac{1}{2}t_1 + t_2 + \frac{1}{2}t_3 \\ \frac{1}{2}t_3 + t_4 + \frac{1}{2}t_5 \\ \frac{1}{2}t_5 + t_6 + \frac{1}{2}t_7 \\ \frac{1}{2}t_7 + t_8 + \frac{1}{2}t_9 \end{pmatrix}.$$

Eliminate  $q$  and  $\frac{dv}{dx}$  from Equations 15, 19 and 30

$$\therefore \frac{dH}{dx} - a_{2,1} u - a_{2,3} \theta = 0 \quad (31)$$

where

$$a_{2,1} = \Delta_q k_n \Delta_q^T \quad (32)$$

and

$$a_{2,3} = -\Delta_q k_n \bar{t} \quad (33)$$



Eliminate  $V$  from Equations 22 and 23a.

$$\therefore \quad \bar{t}_s^T \frac{dq}{dx} - \bar{c}^T \frac{d^2M}{dx^2} + t_l^T \frac{dq}{dx} = 0, \quad (34)$$

since  $\bar{c}^T M_{xq} = \bar{t}_s^T$ . Eliminate  $M$  from Equations 25 and 34.

$$\text{Then,} \quad \bar{t}^T \frac{dq}{dx} - K_I \frac{d^4v}{dx^4} = 0 \quad (35)$$

$$\text{where } K_I = (EI)_1 + (EI)_3 + (EI)_5 + (EI)_7 + (EI)_9. \quad (36)$$

Therefore, from Equations 15, 15a, 15b, and 35

$$\bar{t}^T \frac{dq}{dx} - K_I \frac{d\phi}{dx} = 0. \quad (37)$$

Eliminate  $q$  from Equations 30 and 37. Then,

$$\frac{d\phi}{dx} - K_I^{-1} a_{2,3}^T \frac{du}{dx} - a_{5,4} \frac{d^2v}{dx^2} = 0 \quad (38)$$

$$\text{where} \quad a_{5,4} = K_I^{-1} \bar{t}^T k_n \bar{t}. \quad (38a)$$

Therefore, from Equations 15, 15a, 28 and 38,

$$\frac{d\phi}{dx} - a_{5,2}^H - a_{5,4} \beta = 0, \quad (39)$$

$$\text{where} \quad a_{5,2} = K_I^{-1} a_{2,3}^T a_{1,2}. \quad (40)$$

Equations 15, 15a, 15b, 28, 31 and 39 are a set of 14 linear, ordinary, first order, homogeneous differential equations with constant coefficients, in the 14 dependent variables  $u_i$ ,  $H_i$ ,  $v$ ,  $\theta$ ,  $\beta$  and  $\phi$ . The number of variables is 14 since the matrices  $u$  and  $H$  each contain five variables. The theory of differential equations shows that, for a set of equations of this kind,  $n$  linearly independent solutions exist, where  $n$  is the number of equations, equal to 14 in this case. See Reference 5, article 12-8.

### Matric Differential Equation

From Equations 15, 15a, 15b, 28, 31 and 39,

$$\left\{ \begin{array}{l} du/dx \\ dH/dx \\ dv/dx \\ d\theta/dx \\ d\beta/dx \\ d\phi/dx \end{array} \right\} - \left[ \begin{array}{c|c|c|c|c} a_{1,2} & & & & \\ a_{2,1} & a_{2,3} & & & \\ & 1 & & & \\ & & 1 & & \\ & & & 1 & \\ a_{5,2} & & a_{5,4} & & \end{array} \right] \left\{ \begin{array}{l} u \\ H \\ v \\ \theta \\ \beta \\ \phi \end{array} \right\} = 0 \quad (41)$$

$\frac{dY}{dx} \qquad \qquad \qquad A \qquad \qquad \qquad Y$

The matrices are denoted according to the symbols written under the equation. The symbol  $Y$  denotes a column matrix of beam responses,  $dY/dx$  is its derivative with respect to  $x$ , and  $A$  is a square matrix of constant coefficients.

$$\therefore \frac{dY}{dx} - AY = 0 \quad (42)$$

This linear matric differential equation can be solved by taking

$$Y = Ge^{\lambda x} \quad (43)$$

where  $G$  is an unknown column matrix,  $\lambda$  is an unknown scalar and  $e$  is the base of natural logarithms. Eliminating  $Y$  from Equations 42 and 43 gives

$$(A - \lambda I)G = 0 \quad (44)$$

Evidently,  $\lambda$  and  $G$  are an eigenvalue and an eigenvector of the characteristic Equation 44. Therefore the solution of Equation 42 can be written

$$Y = \sum_{k=1}^{14} C_k G_k e^{\lambda_k x} \quad (45)$$

where  $G_k$ , and  $\lambda_k$  are the  $k$ th eigenvector and eigenvalue of Equation 44, and the  $C_k$ 's are arbitrary constants needed to satisfy the boundary conditions.

### Nature of the Roots of the Characteristic Equation

The eigenvalues (roots) of Equation 44 are expected to be all real, from the nature of the physical problem. The roots can be easily shown to occur in pairs equal in magnitude and opposite in sign, such that if  $\lambda_\alpha$  and  $\lambda_\beta$  are a pair of roots, then  $\lambda_\beta + \lambda_\alpha = 0$ . (See Appendix A.) It is shown subsequently that at least six repeated roots, equal to zero, exist. No more than six such roots are expected, because of the nature of the problem.

### Algebraic Modes

The solutions of Equation 42 of the form given by Equation 43 are here called exponential solutions or exponential modes. The following paragraphs show that other solutions exist which are algebraic functions of  $x$ . Consequently, these solutions are called algebraic solutions or algebraic modes. Six linearly independent algebraic modes are shown to exist. These solutions are also linearly independent of the exponential solutions, since they are algebraic. Therefore, the characteristic equation (Equation 44) can yield at most only eight linearly independent exponential modes. Consequently, six of the roots of Equation 44 must be zero.

In view of the existence of the algebraic modes, the complete solution of Equation 42 can be written in the form

$$Y \approx \sum_{k=1}^8 C_k G_k e^{\lambda_k x} + \sum_{k=1}^6 C_{A_k} Y_{A_k} \quad (46)$$

where the  $C_{A_k}$ 's are arbitrary constants, and the  $Y_{A_k}$ 's (algebraic modes) are column matrices whose elements are algebraic functions of  $x$ . The only requirements for the  $Y_{A_k}$ 's is that they must satisfy Equation 42, and the  $G_k$ 's and  $Y_{A_k}$ 's must be linearly independent. The following  $Y_{A_k}$ 's satisfy these requirements:

Rigid Body Translation Parallel to x -

$$Y_{A_1} = \left\{ \begin{matrix} u(0) \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{matrix} \right\} \left. \begin{matrix} \\ \\ \\ \\ \\ \end{matrix} \right\} 5 \text{ rows} \quad \text{where } u(0) = \left\{ \begin{matrix} 1 \\ 1 \\ 1 \\ 1 \\ 1 \end{matrix} \right\} \quad (47)$$

Uniform Extension Parallel to x -

$$Y_{A_2} = \left\{ u(0)x \mid k_A u(0) \mid 0 \mid 0 \mid 0 \mid 0 \right\} \text{ (Column)} \quad (48)$$

Rigid Body Translation Parallel to z -

$$Y_{A_3} = \left\{ \underbrace{0 \mid 0}_{5 \text{ rows}} \mid \underbrace{1 \mid 0 \mid 0 \mid 0}_{5 \text{ rows}} \right\} \text{ (Column)} \quad (49)$$

Rigid Body Rotation about y -

$$Y_{A_4} = \left\{ u(1) \mid \underbrace{0 \mid x \mid 1 \mid 0 \mid 0}_{5 \text{ rows}} \right\} \text{ (Column)} \quad (50)$$

$$\text{where } u(1) = Q\bar{t} + \alpha u(0) \quad Q = \begin{bmatrix} -1 & -1 & -1 & -1 \\ -1 & -1 & -1 \\ -1 & -1 \\ -1 \\ 0 & 0 & 0 & 0 \end{bmatrix} \quad \alpha = \text{a scalar} \quad (51)$$

Bending about y -

$$Y_{A_5} = \left\{ u(1)x \mid k_A u(1) \mid x^2/2 \mid x \mid 1 \mid 0 \right\} \text{ (Column)} \quad (52)$$

Shear Parallel to z -

$$Y_{A_6} = \left\{ \frac{1}{2}u_{(1)}x^2 + u_{(2)} \mid k_A u_{(1)}x \mid \frac{x^3}{6} \mid \frac{x^2}{2} \mid x \mid 1 \right\} \text{ (Column)} \quad (53)$$

where  $u_{(2)}$  = a column matrix with 5 rows.

$\alpha$  and  $u_{(2)}$  must satisfy the equation

$$Su_{(2)} - T\alpha = W \quad (54)$$

$$\text{where } S = \Delta_q k_n \Delta_q^T, \quad T = k_A u_{(0)}, \quad W = k_A Q \bar{t}. \quad (55)$$

The matrix  $S$  is singular. Equation 54 is a matrix equation equivalent to five scalar simultaneous equations in the five unknown elements of  $u_{(2)}$  and the unknown  $\alpha$ . However one element of  $u_{(2)}$  may be chosen arbitrarily since  $S$  has one degree of singularity. In this respect  $u_{(2)}$  has the character of an eigenvector. Partition  $S$ ,  $T$ ,  $W$  and  $u_{(2)}$ :

$$S = \underbrace{\begin{bmatrix} S_{1,1} & S_{1,2} \\ S_{2,1} & S_{2,2} \end{bmatrix}}_{\substack{1 \text{ col. } 4 \text{ col.}}} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad T = \left\{ \begin{array}{l} T_1 \\ T_2 \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad W = \left\{ \begin{array}{l} W_1 \\ W_2 \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad (56)$$

$$u_{(2)} = \left\{ \begin{array}{l} 0 \\ \bar{u}_{(2)} \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad (57)$$

where the element in the first row of  $u_{(2)}$  has been chosen equal to zero.

From Equations 54, 56 and 57,

$$\begin{cases} S_{1,2} \bar{u}_{(2)} - T_1 \alpha = W_1 \\ S_{2,2} \bar{u}_{(2)} - T_2 \alpha = W_2 \end{cases} \quad (58)$$



$$\therefore \alpha = \frac{1}{T_1} [S_{1,2} \bar{u}_{(2)} - W_1] \quad (59)$$

$$\text{and } \bar{u}_{(2)} = (S_{2,2} - \frac{1}{T_1} T_2 S_{1,2})^{-1} (W_2 - \frac{1}{T_1} T_2 W_1) \quad (60)$$

#### Nature of the Algebraic Solutions

The first five algebraic solutions ( $Y_{A1}, Y_{A2}, \dots, Y_{A5}$ ) correspond to solutions provided by the engineering theory of tension and bending of beams. These solutions have been appropriately named to reflect this fact. Thus  $Y_{A1}$  can be recognized as corresponding to a rigid body translation of the beam parallel to  $x$ , since the longitudinal motions of the centerlines of the structural plies are all equal to 1, and all other responses are zero. The mode  $Y_{A4}$  can be recognized as a rigid body rotation about the centerline of the bottom structural ply, represented by  $Q\bar{\epsilon}$ , plus a rigid body translation parallel to  $x$ , represented by  $\alpha u_{(0)}$ . The mode  $Y_{A6}$  corresponds to the responses calculated by engineering theory when the beam is subjected to a constant shear load. However the responses given by elementary theory involve an approximation, whereas the responses given by  $Y_{A6}$  exactly satisfy the differential equations.

#### Response

Appendix A shows that the eigenvalues  $\lambda_k$  occur in pairs, such that for a pair  $\lambda_\alpha$  and  $\lambda_\beta$ ,

$$\lambda_\beta = -\lambda_\alpha \quad (61)$$

Appendix A also shows that the corresponding eigenvectors have the property that if

$$G_{\alpha} = \begin{pmatrix} G_{u_{\alpha}} \\ G_{H_{\alpha}} \\ G_{v_{\alpha}} \\ G_{\theta_{\alpha}} \\ G_{\beta_{\alpha}} \\ G_{\phi_{\alpha}} \end{pmatrix}, \text{ then } G_{\beta} = \begin{pmatrix} G_{u_{\alpha}} \\ -G_{H_{\alpha}} \\ -G_{v_{\alpha}} \\ G_{\theta_{\alpha}} \\ -G_{\beta_{\alpha}} \\ G_{\phi_{\alpha}} \end{pmatrix} \quad (62)$$

where  $G_{u_{\alpha}}$ ,  $G_{H_{\alpha}}$  etc. are partitions of  $G_{\alpha}$  corresponding to the partitions  $u, H$  etc. of the matrix  $Y$  (Equation 41). Because half of the eigenvalues  $\lambda$  are positive, the corresponding functions  $e^{\lambda x}$  can become very large as  $x$  increases. To avoid this difficulty, proceed as follows: See Equation 46. Let

$$Y_E = \sum_{k=1}^8 C_k G_k e^{\lambda_k x} \quad (\text{exponential response})$$

$$= Y_{El} + Y_{Eg} \quad (63)$$

where

$$Y_{El} = \sum_{k=1}^4 C_{l_k} G_{l_k} e^{\lambda_{l_k} x}, \quad Y_{Eg} = \sum_{k=1}^4 \bar{C}_{g_k} G_{g_k} e^{\lambda_{g_k} x}, \quad (64)$$

and where  $\lambda_{l_k}$  and  $\lambda_{g_k}$  are a pair of eigenvalues having the properties  $\lambda_{l_k} = -\lambda_{g_k}$ ,  $\lambda_{g_k} > 0$ .  $G_{l_k}$  and  $G_{g_k}$  are corresponding eigenvectors. Now

$$Y_{Eg} = \sum_{k=1}^4 \bar{C}_{g_k} G_{g_k} e^{\lambda_{g_k}(x-l+l)} \quad (65)$$

$$= \sum_{k=1}^4 \bar{C}_{g_k} G_{g_k} e^{-\lambda_{g_k}(l-x)} e^{\lambda_{g_k} l} \quad (66)$$

$$= \sum_{k=1}^4 C_{g_k} G_{g_k} e^{\lambda_{l_k}(l-x)} \quad (67)$$

where  $C_{g_k} = \bar{C}_{g_k} e^{\lambda_{g_k} l}$ . (68)

From Equation 46:

$$Y = \sum_{k=1}^4 G_{\ell_k} e^{\lambda_{\ell_k} x} C_{\ell_k} + \sum_{k=1}^4 G_{g_k} e^{\lambda_{g_k}(\ell-x)} C_{g_k} + \sum_{k=1}^6 C_{A_k} Y_{A_k}. \quad (69)$$

Thus the troublesome eigenvalues  $\lambda_{g_k}$ , which are greater than zero, have been lumped into the arbitrary constants  $C_{g_k}$ , and thus eliminated. The functions  $e^{\lambda_{\ell_k} x}$  and  $e^{\lambda_{\ell_k}(\ell-x)}$  are less than or equal to 1 for  $0 < x < \ell$ , since  $\lambda_{\ell_k} < 0$ . Let

$$F_{\ell_k}(x) = e^{\lambda_{\ell_k} x} \quad \text{and} \quad F_{g_k}(x) = e^{\lambda_{g_k}(\ell-x)} \quad (70)$$

$$\therefore Y = \sum_{k=1}^4 G_{\ell_k} F_{\ell_k}(x) C_{\ell_k} + \sum_{k=1}^4 G_{g_k} F_{g_k}(x) C_{g_k} + \sum_{k=1}^6 C_{A_k} Y_{A_k} \quad (71)$$

$$\therefore Y(x) = H_{\ell} F_{\ell D}(x) C_{\ell} + H_g F_{g D}(x) C_g + Y_A(x) C_A \quad (72)$$

where the symbols in this equation are defined by Equations 73, 74, 75 and 7

$$H_{\ell} = \begin{bmatrix} G_{\ell_1} & G_{\ell_2} & G_{\ell_3} & G_{\ell_4} \end{bmatrix} \quad H_g = \begin{bmatrix} G_{g_1} & G_{g_2} & G_{g_3} & G_{g_4} \end{bmatrix} \quad (73)$$

$$F_{\ell}(x) = \begin{Bmatrix} F_{\ell_1}(x) \\ F_{\ell_2}(x) \\ F_{\ell_3}(x) \\ F_{\ell_4}(x) \end{Bmatrix} \quad F_g(x) = \begin{Bmatrix} F_{g_1}(x) \\ F_{g_2}(x) \\ F_{g_3}(x) \\ F_{g_4}(x) \end{Bmatrix} \quad (74)$$

$F_{\ell D}(x)$ ,  $F_{g D}(x)$  =  $F_{\ell}(x)$ ,  $F_g(x)$  diagonalized

$$C_{\ell} = \begin{Bmatrix} C_{\ell_1} \\ C_{\ell_2} \\ C_{\ell_3} \\ C_{\ell_4} \end{Bmatrix} \quad C_g = \begin{Bmatrix} C_{g_1} \\ C_{g_2} \\ C_{g_3} \\ C_{g_4} \end{Bmatrix} \quad C_A = \begin{Bmatrix} C_{A_1} \\ C_{A_2} \\ \vdots \\ C_{A_6} \end{Bmatrix} \quad (75)$$

$$Y_A(x) = \left[ Y_{A_1}(x) \mid Y_{A_2}(x) \mid \dots \mid Y_{A_6}(x) \right] . \quad (76)$$

Partition  $Y(x)$ ,  $H_\ell$ ,  $H_g$ ,  $Y_A(x)$  :

$$\begin{pmatrix} u(x) \\ H(x) \\ v(x) \\ \theta(x) \\ \beta(x) \\ \phi(x) \end{pmatrix} = \begin{pmatrix} H_{u\ell} \\ H_{H\ell} \\ H_{v\ell} \\ H_{\theta\ell} \\ H_{\beta\ell} \\ H_{\phi\ell} \end{pmatrix} F_{\ell D}(x) C_\ell + \begin{pmatrix} H_u \\ -H_{H\ell} \\ -H_{v\ell} \\ H_{\theta\ell} \\ -H_{\beta\ell} \\ H_{\phi\ell} \end{pmatrix} F_{gD}(x) C_g + \begin{pmatrix} Y_{Au}(x) \\ Y_{AH}(x) \\ Y_{Av}(x) \\ Y_{A\theta}(x) \\ Y_{AB}(x) \\ Y_{A\phi}(x) \end{pmatrix} C_A \quad (77)$$

$Y(x)$ 
 $H_\ell$ 
 $H_g$ 
 $Y_A(x)$

The matrices are denoted according to the symbols written under the equation. The partitions of  $Y_A(x)$  are defined, from Equation 76, by the following equations:

$$Y_{Au}(x) = \left[ u(0) \mid u(0)x \mid 0 \mid u(1) \mid u(1)x \mid \frac{1}{2}u(1)x^2 + u(2) \right] \quad (78)$$

$$Y_{AH}(x) = \left[ 0 \mid k_A u(0) \mid 0 \mid 0 \mid k_A u(1) \mid k_A u(1)x \right] \quad (79)$$

$$Y_{Av}(x) = \left[ 0 \mid 0 \mid 1 \mid x \mid x^2/2 \mid x^3/6 \right] \quad (80)$$

$$Y_{A\theta}(x) = \left[ 0 \mid 0 \mid 0 \mid 1 \mid x \mid x^2/2 \right] \quad (81)$$

$$Y_{AB}(x) = \left[ 0 \mid 0 \mid 0 \mid 0 \mid 1 \mid x \right] \quad (82)$$

$$Y_{A\phi}(x) = \left[ 0 \mid 0 \mid 0 \mid 0 \mid 0 \mid 1 \right] \quad (83)$$

# Interlayer Shear Flows, Structural Ply Shears and Moments

From Equations 15 and 30

$$q = k_n \bar{t} \theta - k_n \Delta_q^T u \quad (84)$$

From Equations 15b, 22 and 25,

$$V = V_q q - k_I \phi \quad (85)$$

where  $V_q = M_{xq}$  .

From Equations 15a and 25

$$M = k_I \beta \quad (86)$$

Let  $S(x)$  = total shear on the cross section at station  $x$ .

$$\therefore S = \begin{bmatrix} 1 & 1 & 1 & 1 & 1 \end{bmatrix} \begin{Bmatrix} v_1 \\ v_3 \\ v_5 \\ v_7 \\ v_9 \end{Bmatrix} + \begin{bmatrix} t_2 & t_4 & t_6 & t_8 \end{bmatrix} \begin{Bmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \end{Bmatrix} \quad (87)$$

$u_{(0)}^T$ 
 $V$ 
 $t_\ell^T$ 
 $q$

The matrices are denoted according to the symbols written under the equation.

Therefore  $S = u_{(0)}^T V + t_\ell^T q \quad (88)$

Eliminate  $V$  from Equations 85 and 88,

$$\therefore S = \bar{t}^T q - K_I \phi \quad (89)$$

Eliminate  $q$  from Equations 84 and 89,

$$\therefore S = a_{2,3}^T u + K_I a_{5,4} \theta - K_I \phi \quad (90)$$



### Boundary Conditions

Figure 8 shows the boundary conditions for a fixed ended beam.

At  $x = 0$  —

$$\begin{cases} u(0) = 0 & (5 \text{ conditions}) \\ v(0) = 0 & (1 \text{ condition}) \\ \theta(0) = 0 & (1 \text{ condition}) \end{cases}$$

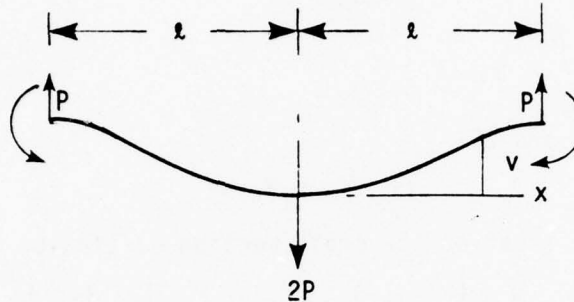


Figure 8. Boundary conditions.

For all  $x$  —

The total shear on the cross section is constant

$$\therefore S(x) = P.$$

$S(x)$  is most easily evaluated at  $x = 0$ , therefore take

$$S(0) = P \quad (1 \text{ condition}).$$

From Equation 90,

$$-K_I \phi(0) = P \quad \text{and} \quad \phi(0) = -K_I^{-1} P, \quad (92)$$

since  $u(0) = \theta(0) = 0$ .

Condition 1. Fixed Ends at  $x = \pm l$ . For  $x = l$  —

$$\begin{cases} u(l) = 0 & (5 \text{ conditions}) \\ \theta(l) = 0 & (1 \text{ condition}) \end{cases} \quad (93)$$

From Equation 77

$$\left. \begin{aligned} H_{u\ell} F_{\ell D}(0) C_\ell + H_{u\ell} F_{gD}(0) C_g + Y_{Au}(0) C_A &= 0 \\ H_{v\ell} F_{\ell D}(0) C_\ell - H_{v\ell} F_{gD}(0) C_g + Y_{Av}(0) C_A &= 0 \\ H_{\theta\ell} F_{\ell D}(0) C_\ell + H_{\theta\ell} F_{gD}(0) C_g + Y_{A\theta}(0) C_A &= 0 \\ H_{\phi\ell} F_{\ell D}(0) C_\ell + H_{\phi\ell} F_{gD}(0) C_g + Y_{A\phi}(0) C_A &= -K_I^{-1} P \\ H_{u\ell} F_{\ell D}(\ell) C_\ell + H_{u\ell} F_{gD}(\ell) C_g + Y_{Au}(\ell) C_A &= 0 \\ H_{\theta\ell} F_{\ell D}(\ell) C_\ell + H_{\theta\ell} F_{gD}(\ell) C_g + Y_{A\theta}(\ell) C_A &= 0 \end{aligned} \right\} \quad (94)$$

$$\therefore \left[ \begin{array}{ccc|ccc} H_{u\ell} F_{\ell D}(0) & H_{u\ell} F_{gD}(0) & Y_{Au}(0) & C_\ell & 0 \\ H_{v\ell} F_{\ell D}(0) & -H_{v\ell} F_{gD}(0) & Y_{Av}(0) & C_g & 0 \\ H_{\theta\ell} F_{\ell D}(0) & H_{\theta\ell} F_{gD}(0) & Y_{A\theta}(0) & C_A & 0 \\ H_{\phi\ell} F_{\ell D}(0) & H_{\phi\ell} F_{gD}(0) & Y_{A\phi}(0) & & -K_I^{-1} P \\ H_{u\ell} F_{\ell D}(\ell) & H_{u\ell} F_{gD}(\ell) & Y_{Au}(\ell) & & 0 \\ H_{\theta\ell} F_{\ell D}(\ell) & H_{\theta\ell} F_{gD}(\ell) & Y_{A\theta}(\ell) & & 0 \end{array} \right] \begin{matrix} C \\ \psi \end{matrix} = \begin{matrix} 0 \\ 0 \\ 0 \\ -K_I^{-1} P \\ 0 \\ 0 \end{matrix} \quad (95)$$

The matrices are denoted according to the symbols written under the equation. The symbol C denotes a column matrix of arbitrary constants,  $\psi$  is a column matrix of boundary conditions, and B is a coefficient matrix.

$$\therefore BC = \psi \quad (96)$$

From Equations 70 and 74

$$F_{\ell D}(0) = F_{gD}(\ell) = I \quad (97)$$

$$\text{and } F_{\ell D}(\ell) = F_{gD}(0) \quad (98)$$

where  $I$  is a unit (identity) matrix. Therefore;  $B =$

$$\begin{bmatrix} H_{u\ell} & H_{u\ell}F_{\ell D}(\ell) & u(0) & 0 & 0 & u(1) & 0 & u(2) \\ H_{v\ell} & -H_{v\ell}F_{\ell D}(\ell) & 0 & 0 & 1 & 0 & 0 & 0 \\ H_{\theta\ell} & H_{\theta\ell}F_{\ell D}(\ell) & 0 & 0 & 0 & 1 & 0 & 0 \\ H_{\phi\ell} & H_{\phi\ell}F_{\ell D}(\ell) & 0 & 0 & 0 & 0 & 0 & 1 \\ H_{u\ell}F_{\ell D}(\ell) & H_{u\ell} & u(0) & u(0)\ell & 0 & u(1) & u(1)\ell & \frac{1}{2}u(1)\ell^2 + u(2) \\ H_{\theta\ell}F_{\ell D}(\ell) & H_{\theta\ell} & 0 & 0 & 0 & 1 & \ell & \frac{\ell^2}{2} \end{bmatrix}$$

... (99)

### Effective Beam Stiffness

For a fixed-ended monolithic beam carrying a concentrated load at the center,

$$v(\ell) = \frac{P\ell^3}{12EI} \quad . \quad (100)$$

Therefore define the effective stiffness of the laminated beam as

$$EI_{\text{eff}} = \frac{P\ell^3}{12v(\ell)} \quad . \quad (101)$$

### EQUATION SUMMARY

This section summarizes the equations for the laminated beam.  
Figure 8a shows the basic beam dimensions.

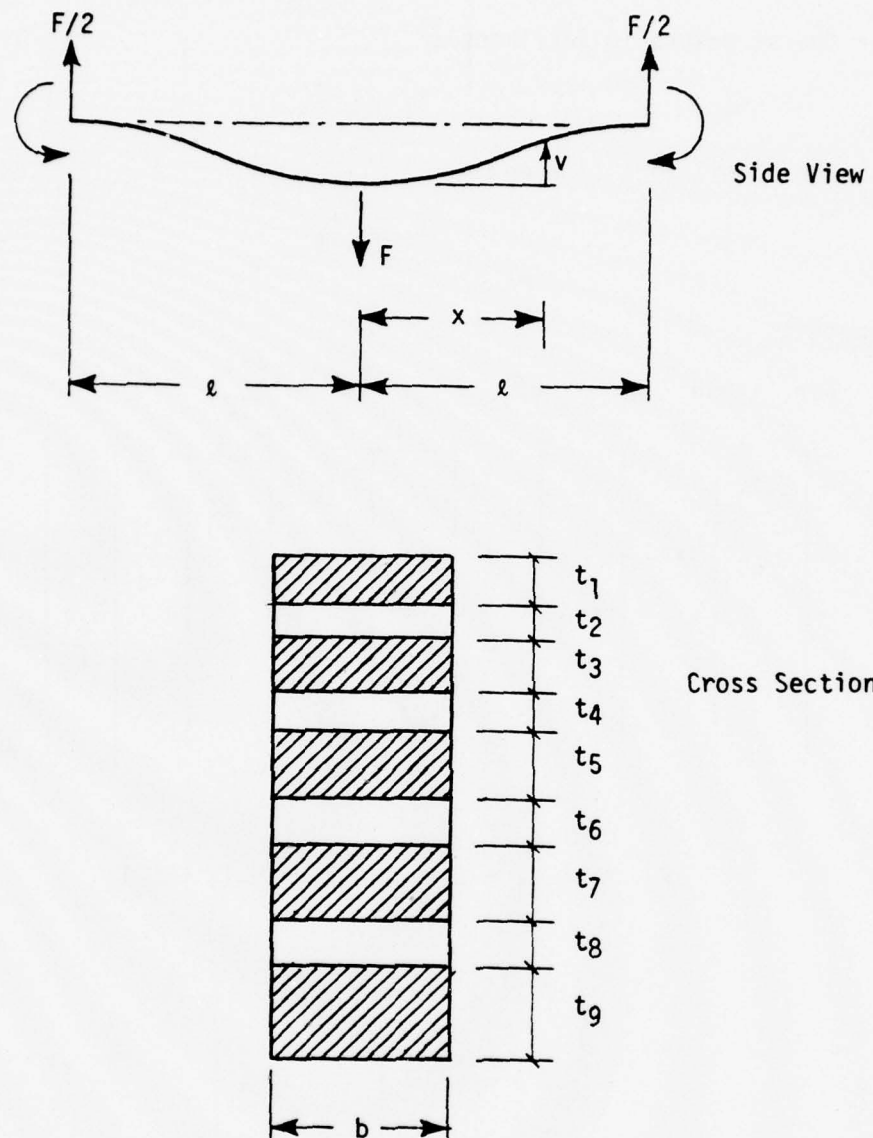


Figure 8a. Beam dimensions.

Odd numbered layers are structural plies.  
Even numbered layers are interlayers.

### Input

$F, \ell, b, x = x_j, j = 0, 1, 2, \dots, n_x$  where  $x_0 = 0, x_{n_x} = \ell$ .

For the structural plies (i odd):

$$t_i, E_i$$

For the interlayers (i even):

$$t_i, G_i$$

### Equations

$$\text{For } i \text{ odd: } A_i = bt_i \quad I_i = \frac{bt_i^3}{12} \quad (102)$$

$$k_A = \begin{bmatrix} E_1 A_1 & & & & \\ & E_3 A_3 & & & \\ & & E_5 A_5 & & \\ & & & E_7 A_7 & \\ & & & & E_9 A_9 \end{bmatrix} \quad k_n = \begin{bmatrix} \frac{G_2 b}{t_2} & & & & \\ & \frac{G_4 b}{t_4} & & & \\ & & \frac{G_6 b}{t_6} & & \\ & & & \frac{G_8 b}{t_8} & \end{bmatrix} \quad \dots (103)$$

$$K_I = E_1 I_1 + E_3 I_3 + E_5 I_5 + E_7 I_7 + E_9 I_9 \quad (104)$$

$$\bar{t} = \begin{pmatrix} \frac{1}{2}t_1 + t_2 + \frac{1}{2}t_3 \\ \frac{1}{2}t_3 + t_4 + \frac{1}{2}t_5 \\ \frac{1}{2}t_5 + t_6 + \frac{1}{2}t_7 \\ \frac{1}{2}t_7 + t_8 + \frac{1}{2}t_9 \end{pmatrix} \quad \Delta_q = \begin{bmatrix} -1 & & & \\ 1 & -1 & & \\ & 1 & -1 & \\ & & 1 & -1 \\ & & & 1 \end{bmatrix} \quad (105)$$



$$\left. \begin{aligned}
 a_{1,2} &= k_A^{-1} \\
 a_{2,1} &= \Delta_q k_n \Delta_q^T \\
 a_{2,3} &= -\Delta_q k_n \bar{t} \\
 a_{5,2} &= K_I^{-1} a_{2,3}^T a_{1,2} \\
 a_{5,4} &= K_I^{-1} \bar{t}^T k_n \bar{t}
 \end{aligned} \right\} \quad (106)$$

$$A =_{14 \times 14} \begin{bmatrix} 0 & a_{1,2} & 0 & 0 & 0 & 0 \\ a_{2,1} & 0 & 0 & a_{2,3} & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \\ 0 & a_{5,2} & 0 & 0 & a_{5,4} & 0 \end{bmatrix} \quad (107)$$

Find the eigenvalues and eigenvectors of

$$(A - \lambda I)G = 0 \quad (108)$$

Six of the eigenvalues should be zero. The remaining eight eigenvalues should be real and should occur in pairs such that for any pair,  $\lambda_\ell$  and  $\lambda_g$ ,

$$\lambda_\ell < 0, \quad \lambda_g > 0 \quad \text{and} \quad \lambda_\ell + \lambda_g = 0 \quad (109)$$

If the eigenvalues do not occur in this manner, print an error message. Otherwise, discard the eigenvalues which are equal to or greater than zero, and the corresponding eigenvectors. Denote the remaining eigenvalues and eigenvectors  $\lambda_{\ell_i}$  and  $G_{\ell_i}$ ,  $i = 1, 2, 3, 4$ .

Form

$$H_\ell = \left[ G_{\ell 1} \mid G_{\ell 2} \mid G_{\ell 3} \mid G_{\ell 4} \right] F_\ell(\ell) = \begin{pmatrix} \lambda_{\ell 1}^\ell \\ e \\ \lambda_{\ell 2}^\ell \\ e \\ \lambda_{\ell 3}^\ell \\ e \\ \lambda_{\ell 4}^\ell \\ e \end{pmatrix} \quad (110)$$

$F_{\ell D}(\ell) = F_\ell(\ell)$  diagonalized. Partition  $H_\ell$  :

$$H_\ell = \begin{bmatrix} H_{u\ell} \\ H_{H\ell} \\ H_{V\ell} \\ H_{\theta\ell} \\ H_{\beta\ell} \\ H_{\phi\ell} \end{bmatrix} \text{ Form } u(0) = \begin{pmatrix} 1 \\ 1 \\ 1 \\ 1 \\ 1 \end{pmatrix} \quad Q = \begin{bmatrix} -1 & -1 & -1 & -1 \\ & -1 & -1 & -1 \\ & & -1 & -1 \\ & & & -1 \\ 0 & 0 & 0 & 0 \end{bmatrix} \quad (111)$$

$$S = \Delta_q k_n \Delta_q^T ; \quad T = k_A u(0) ; \quad W = k_A Q \bar{e} . \quad (112)$$

Partition  $S$ ,  $T$  and  $W$  :

$$S = \underbrace{\begin{bmatrix} S_{1,1} & S_{1,2} \\ S_{2,1} & S_{2,2} \end{bmatrix}}_{\substack{1 \text{ col.} \quad 4 \text{ cols.}}} \left\{ \begin{array}{l} 1 \text{ row,} \\ 4 \text{ rows} \end{array} \right. \quad T = \left\{ \begin{array}{l} T_1 \\ T_2 \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row,} \\ 4 \text{ rows} \end{array} \right. \quad W = \left\{ \begin{array}{l} W_1 \\ W_2 \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right. \quad \dots (113)$$

Discard  $S_{1,1}$  and  $S_{2,1}$  .

$$\bar{u}_{(2)} = (S_{2,2} - \frac{1}{T_1} T_2 S_{1,2})^{-1} (W_2 - \frac{1}{T_1} T_2 W_1) \quad (114)$$

$$\alpha = \frac{1}{T_1} \left[ S_{1,2} \bar{u}_{(2)} - W_1 \right] \quad (115)$$

$$u_{(1)} = Q\bar{t} + \alpha u_{(0)} \quad (116)$$

$$u_{(2)} = \begin{Bmatrix} 0 \\ \bar{u}_{(2)} \end{Bmatrix} \quad (117)$$

Form

$$B = \begin{bmatrix} H_{u\ell} & H_{u\ell} F_{\ell D}(\ell) & u_{(0)} & 0 & 0 & u_{(1)} & 0 & u_{(2)} \\ H_{v\ell} & -H_{v\ell} F_{\ell D}(\ell) & 0 & 0 & 1 & 0 & 0 & 0 \\ H_{\theta\ell} & H_{\theta\ell} F_{\ell D}(\ell) & 0 & 0 & 0 & 1 & 0 & 0 \\ H_{\phi\ell} & H_{\phi\ell} F_{\ell D}(\ell) & 0 & 0 & 0 & 0 & 0 & 1 \\ H_{u\ell} F_{\ell D}(\ell) & H_{u\ell} & u_{(0)} & u_{(0)}^{\ell} & 0 & u_{(1)} & u_{(1)}^{\ell} & \frac{1}{2} u_{(1)}^{\ell^2+u_{(2)}} \\ H_{\theta\ell} F_{\ell D}(\ell) & H_{\theta\ell} & 0 & 0 & 0 & 1 & \ell & \frac{\ell^2}{2} \end{bmatrix} \quad \dots (118)$$

$$P = F/2, \text{ and} \quad (119)$$

$$\psi = \left\{ 0^* \mid 0 \mid 0 \mid -K_I^{-1}P \mid 0^* \mid 0 \right\} \text{ (column)} \quad (120)$$

$$\text{Solve } BC = \psi \text{ for } C. \quad (121)$$

Form the following for  $x = x_j$ . The values of  $x_j$  are input.

$$\text{Then } F_{\ell_k}(x_j) = e^{\lambda_{\ell_k} x_j} \text{ and } F_{g_k}(x) = e^{\lambda_{\ell_k}(\ell - x_j)}. \quad (122)$$

\* These null matrices each have five rows.

Then

$$F_{\ell}(x_j) = \begin{Bmatrix} F_{\ell_1}(x_j) \\ F_{\ell_2}(x_j) \\ F_{\ell_3}(x_j) \\ F_{\ell_4}(x_j) \end{Bmatrix} \quad F_g(x_j) = \begin{Bmatrix} F_{g_1}(x_j) \\ F_{g_2}(x_j) \\ F_{g_3}(x_j) \\ F_{g_4}(x_j) \end{Bmatrix} \quad (123)$$

$$F_{\ell D}(x_j) \text{ and } F_{gD}(x_j) = F_{\ell}(x_j) \text{ and } F_g(x_j) \text{ diagonalized.} \quad (124)$$

$$\left. \begin{aligned} Y_{Au}(x_j) &= \begin{bmatrix} u(0) & u(0)x_j & 0 & u(1) & u(1)x_j & \frac{1}{2}u(1)x_j^2 + u(2) \end{bmatrix} \\ Y_{AH}(x_j) &= \begin{bmatrix} 0 & k_A^u(0) & 0 & 0 & k_A^u(1) & k_A^u(1)x_j \end{bmatrix} \\ Y_{Av}(x_j) &= \begin{bmatrix} 0 & 0 & 1 & x_j & x_j^2/2 & x_j^3/6 \end{bmatrix} \\ Y_{A\theta}(x_j) &= \begin{bmatrix} 0 & 0 & 0 & 1 & x_j & x_j^2/2 \end{bmatrix} \\ Y_{AB}(x_j) &= \begin{bmatrix} 0 & 0 & 0 & 0 & 1 & x_j \end{bmatrix} \\ Y_{A\phi}(x_j) &= \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \end{aligned} \right\} \quad (125)$$

$$\text{Partition } C : \quad C = \left\{ \begin{array}{l} C_{\ell} \\ C_g \\ C_A \end{array} \right\} \quad \left. \begin{array}{l} 4 \text{ rows} \\ 4 \text{ rows} \\ 6 \text{ rows} \end{array} \right\} \quad (126)$$

$$\begin{Bmatrix} u(x_j) \\ H(x_j) \\ v(x_j) \\ \theta(x_j) \\ \beta(x_j) \\ \phi(x_j) \end{Bmatrix} = \begin{bmatrix} H_{u\ell} \\ H_{H\ell} \\ H_{v\ell} \\ H_{\theta\ell} \\ H_{\beta\ell} \\ H_{\phi\ell} \end{bmatrix} F_{\ell D}(x_j) C_{\ell} + \begin{bmatrix} H_{u\ell} \\ -H_{H\ell} \\ -H_{v\ell} \\ H_{\theta\ell} \\ -H_{\beta\ell} \\ H_{\phi\ell} \end{bmatrix} F_{gD}(x_j) C_g + \begin{bmatrix} Y_{Au}(x_j) \\ Y_{AH}(x_j) \\ Y_{Av}(x_j) \\ Y_{A\theta}(x_j) \\ Y_{AB}(x_j) \\ Y_{A\phi}(x_j) \end{bmatrix} C_A \quad (127)$$

$$q(x_j) = k_n \bar{e}\theta(x_j) - k_n \Delta_q^T u(x_j) \quad (128)$$

$$V_q = \frac{1}{2} \begin{bmatrix} t_1 & & & & \\ & t_3 & & & \\ & & t_5 & & \\ & & & t_7 & \\ & & & & t_9 \end{bmatrix} \quad k_I = \begin{Bmatrix} E_1 I_1 \\ E_3 I_3 \\ E_5 I_5 \\ E_7 I_7 \\ E_9 I_9 \end{Bmatrix} \quad (129)$$

$$V(x_j) = V_q q(x_j) - k_I \phi(x_j) \quad (130)$$

$$M(x_j) = k_I \beta(x_j) \quad (131)$$

$$(EI)_{\text{eff.}} = \frac{p \ell^3}{12v(x_{n_x})} \quad (132)$$

Output (printed)

$u(x_j)$  ,  $H(x_j)$  ,  $v(x_j)$  ,  $\theta(x_j)$  ,  $\beta(x_j)$  ,  $\phi(x_j)$

$q(s_j)$  ,  $V(x_j)$  ,  $M(x_j)$  for  $j = 0, 1, 2 \dots n_x$

$(EI)_{\text{eff}}$



## ILLUSTRATIVE EXAMPLES

Four sample cases have been selected to illustrate the usefulness of this program. Cross sections and material properties for the four cases are shown in Figures 9, 10, 11 and 12. The end conditions have been defined as fixed. The load is one pound and has been applied at the beam center line. The outer glass shield is fixed at the ends, although for a typical windshield the glass floats on an interlayer and is free-floating at the ends. The program is set up to handle a total of nine plies. For beams with less than nine plies, such as Cases B, C and D, small values were assigned to the thickness, modulus of elasticity, and shear modulus to negate the influence of those fictitious plies.

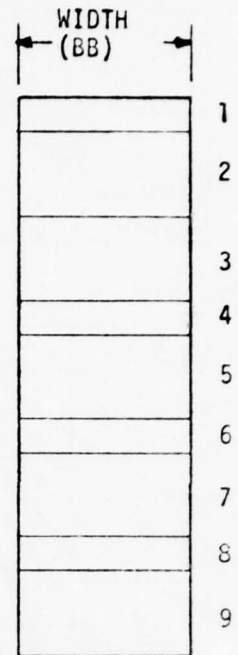
Input data and output data for five examples are presented in Appendix D. Case A was analyzed for a beam ten inches long and for a beam 30 inches long. Case D and Case C were analyzed for a beam ten inches long. Case B was analyzed for a beam 20 inches long. Table 1 is a summary and explanation of input data for Case A. Table 2 is a summary and explanation of output data for Case A. Table 3 lists a summary of deflections for the five examples as well as additional deflections for the four cases at various lengths measured at the beam center line. The deflections do not vary with the cube of the length and this deviation is shown in Figure 13, which compares a laminated beam to a multiply beam without interlayer for Cases A, B, C and D.

At the end of output data for each example in Appendix D, there is a printout for EI(EFF) which is defined as "effective stiffness". This value is calculated from the deflection at the beam center line using the formula for a beam with fixed ends and a center point load which is:

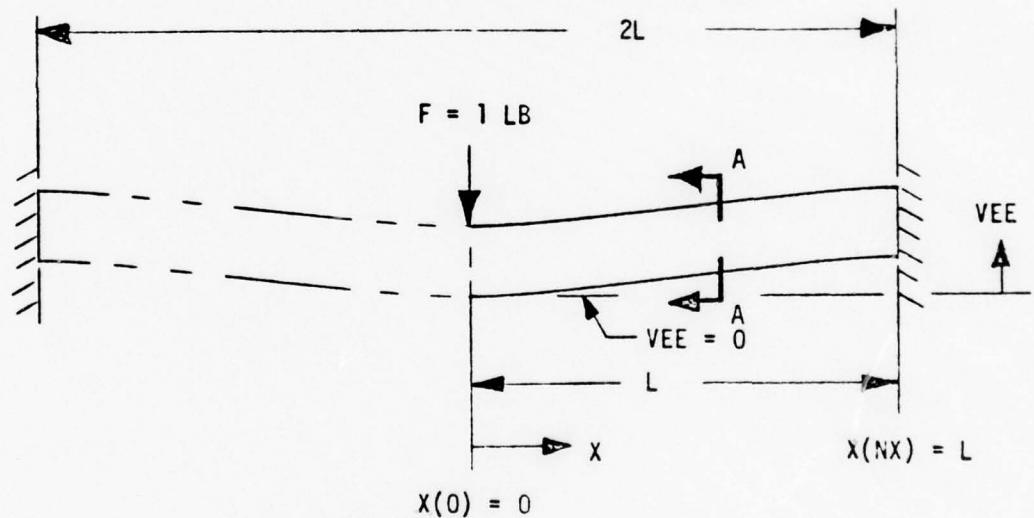
$$EI(EFF) = \frac{FL^3}{192V}$$

CASE A				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.100	$10 \times 10^6$	$4 \times 10^6$	GLASS
2	0.250	183	73	CIP SILICONE
3	0.250	340,000	109,000	POLYCARBONATE
4	0.100	183	73	CIP SILICONE
5	0.250	340,000	109,000	POLYCARBONATE
6	0.100	183	73	CIP SILICONE
7	0.250	340,000	109,000	POLYCARBONATE
8	0.100	183	73	CIP SILICONE
9	0.250	340,000	109,000	POLYCARBONATE



SECTION A-A



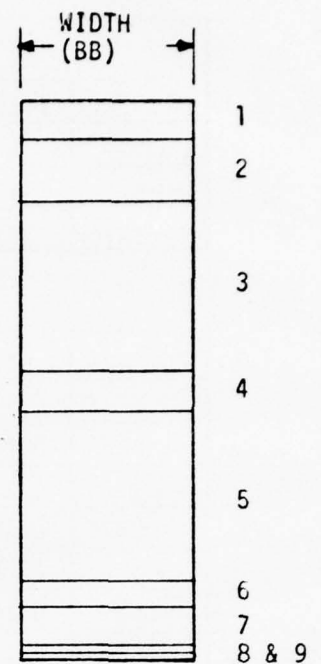
DEFLECTED SHAPE OF BEAM - NO SCALE

See Tables 1 and 2 for symbols

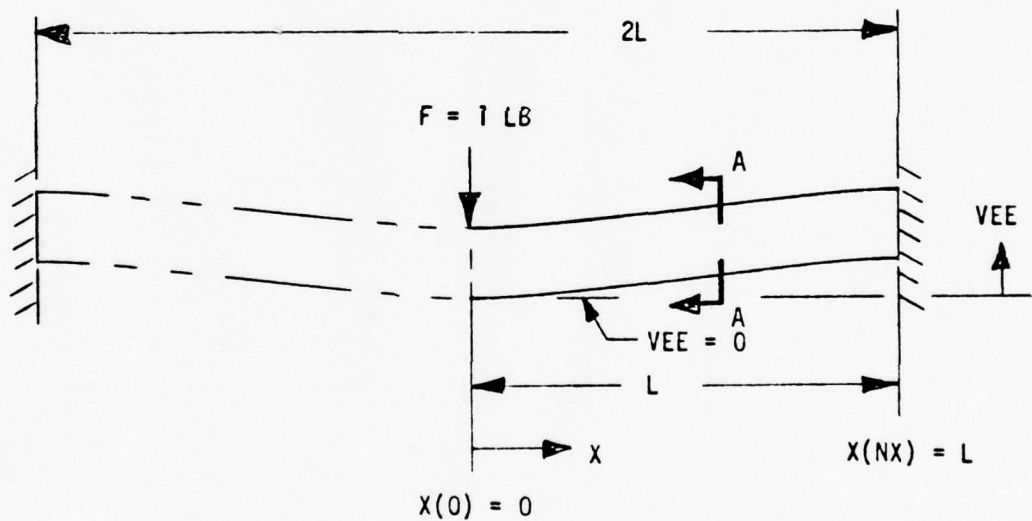
Figure 9. Cross sectional properties for Case A.

CASE B				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.110	$10 \times 10^6$	$4 \times 10^6$	GLASS
2	0.188	1,100	440	PPG 112
3	0.500	$10 \times 10^6$	$4 \times 10^6$	GLASS
4	0.120	1,100	440	PPG 112
5	0.500	$10 \times 10^6$	$4 \times 10^6$	GLASS
6	0.080	1,100	440	PPG 112
7	0.110	$10 \times 10^6$	$4 \times 10^6$	GLASS
8	0.021	1	1	FICTITIOUS
9	0.021	1	1	FICTITIOUS



SECTION A-A



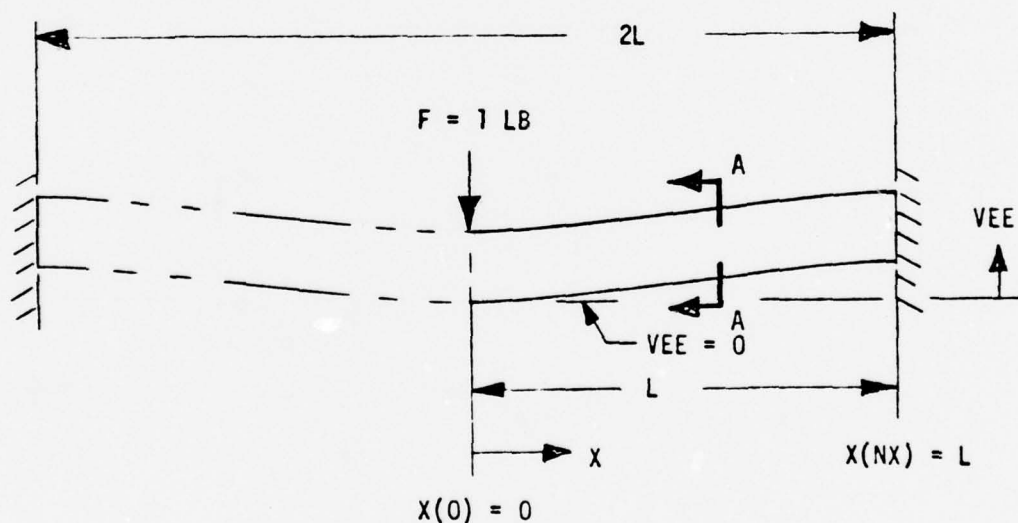
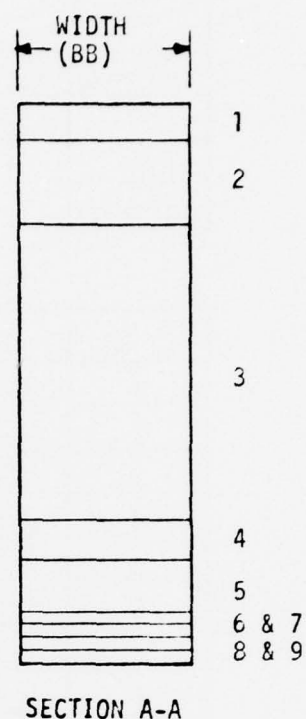
DEFLECTED SHAPE OF BEAM - NO SCALE

See Tables 1 and 2 for symbols

Figure 10. Cross sectional properties for Case B.

CASE C				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.105	$10 \times 10^6$	$4 \times 10^6$	GLASS
2	0.250	183	73	CIP SILICONE
3	0.870	340,000	109,000	POLYCARBONATE
4	0.120	183	73	CIP SILICONE
5	0.150	340,000	109,000	POLYCARBONATE
6	0.03875	1	1	FICTITIOUS
7	0.03875	1	1	FICTITIOUS
8	0.03875	1	1	FICTITIOUS
9	0.03875	1	1	FICTITIOUS



See Tables 1 and 2 for symbols

Figure 11. Cross sectional properties for Case C.

CASE D				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.100	$10 \times 10^6$	$4 \times 10^6$	GLASS
2	0.250	1,100	440	PPG 112
3	0.500	340,000	109,000	POLYCARBONATE
4	0.100	1,100	440	PPG 112
5	0.500	340,000	109,000	POLYCARBONATE
6	0.100	1,100	440	PPG 112
7	0.080	490,000	181,000	ACRYLIC
8	0.010	1	1	FICTITIOUS
9	0.010	1	1	FICTITIOUS

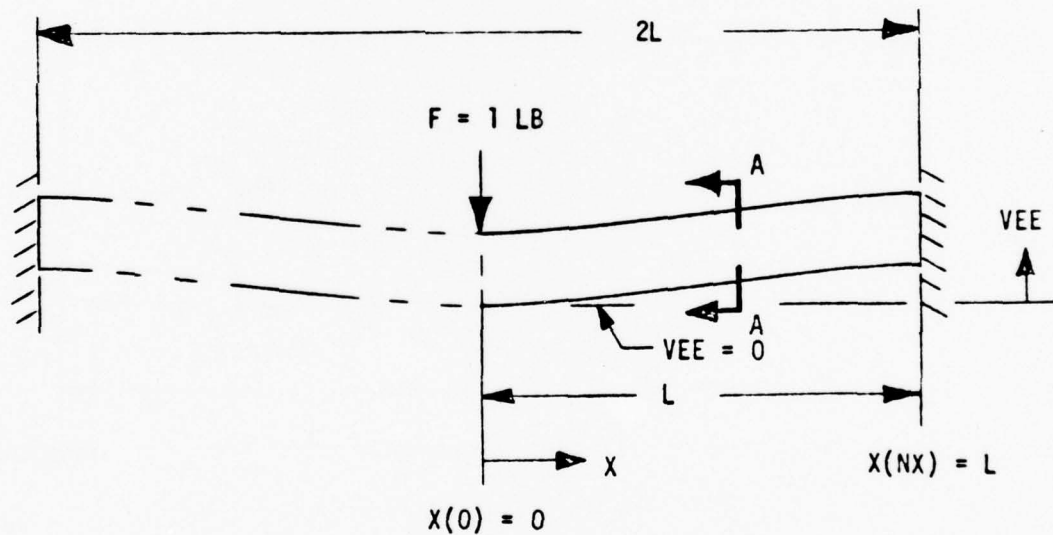
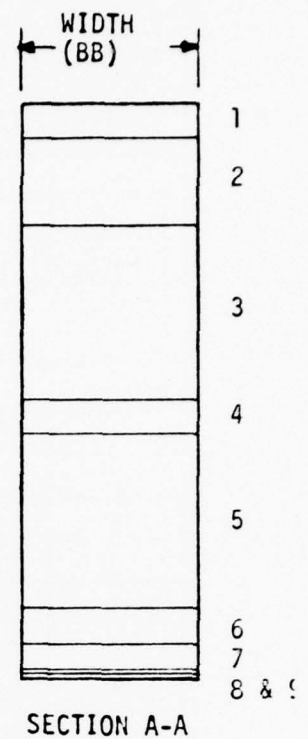


Figure 12. Cross sectional properties for Case D.



TABLE 1. SUMMARY OF INPUT DATA FOR CASE A

COMPUTER PROGRAM SYMBOLS	DESCRIPTION	INPUT DATA
NX	Number of Increments	10.0
F	Load (Pounds)	1.0
L	Half Length (Inches)	5.0
BB	Width (Inches)	1.0
T	Ply Thickness (Inches) (5 Structural Plies and 4 Interlayer Plies)	.10 .25, .25, .10, .25, .10, .25, .10, .25
E	Modulus of Elasticity (PSI)	$10^7$ , 183, 340,000, 183, 340,000, 183, 340,000, 183, 340,000
G	Shear Modulus (PSI)	$4 \times 10^6$ , 73, 109,000, 73, 109,000, 73, 109,000, 73, 109,000
X	Distance from Center of Beam (Inches)	0, .50, 1.0, 1.5, 2.0, 2.5, 3.0, 3.5, 4.0, 4.5, 5.0

TABLE 2. SUMMARY OF OUTPUT DATA FOR CASE A

COMPUTER PROGRAM SYMBOLS	DESCRIPTION	OUTPUT DATA
X(0)	Increment Number	0
U	Horizontal Displacement of Center of Structural Plies (Inches)	0
H	Axial Load in Structural Plies (Pounds)	-.0959, -.0979, -.00192, -.00389, -.1918
VEE	Vertical Deflection (Inches) (Zero Deflection is Point of Maximum Deflection at Center)	0
QXJ	Shear Flow in Interlayer (Pounds Per Inch)	0
VXJ	Shear Force in Ply (Pounds)	.16, .085, .085, .085, .085
MXJ	Moment on Structural Ply (Inch-Pounds)	.322, .171, .171, .171, .171
THETA	Slope (Radians)	-5.551115E-17
BETA	Rate of Change of Slope	.000386
PHI	Rate of Change of BETA	-.000192

TABLE 3. DEFLECTION - (INCHES)

	OUTPUT FROM PROGRAM			
LENGTH OF BEAM - INCHES	10	20	30	50
<u>CASE A</u> 4 Ply Polycarbonate	.00153	.00733	.0152	.0339
<u>CASE B</u> 2-Ply Glass	.0000239	.000175	.000520	.00178
<u>CASE C</u> 1-Ply Polycarbonate	.000252	.00178	.00511	.0172
<u>CASE D</u> 2-Ply Polycarbonate	.000364	.00142	.00292	.00731

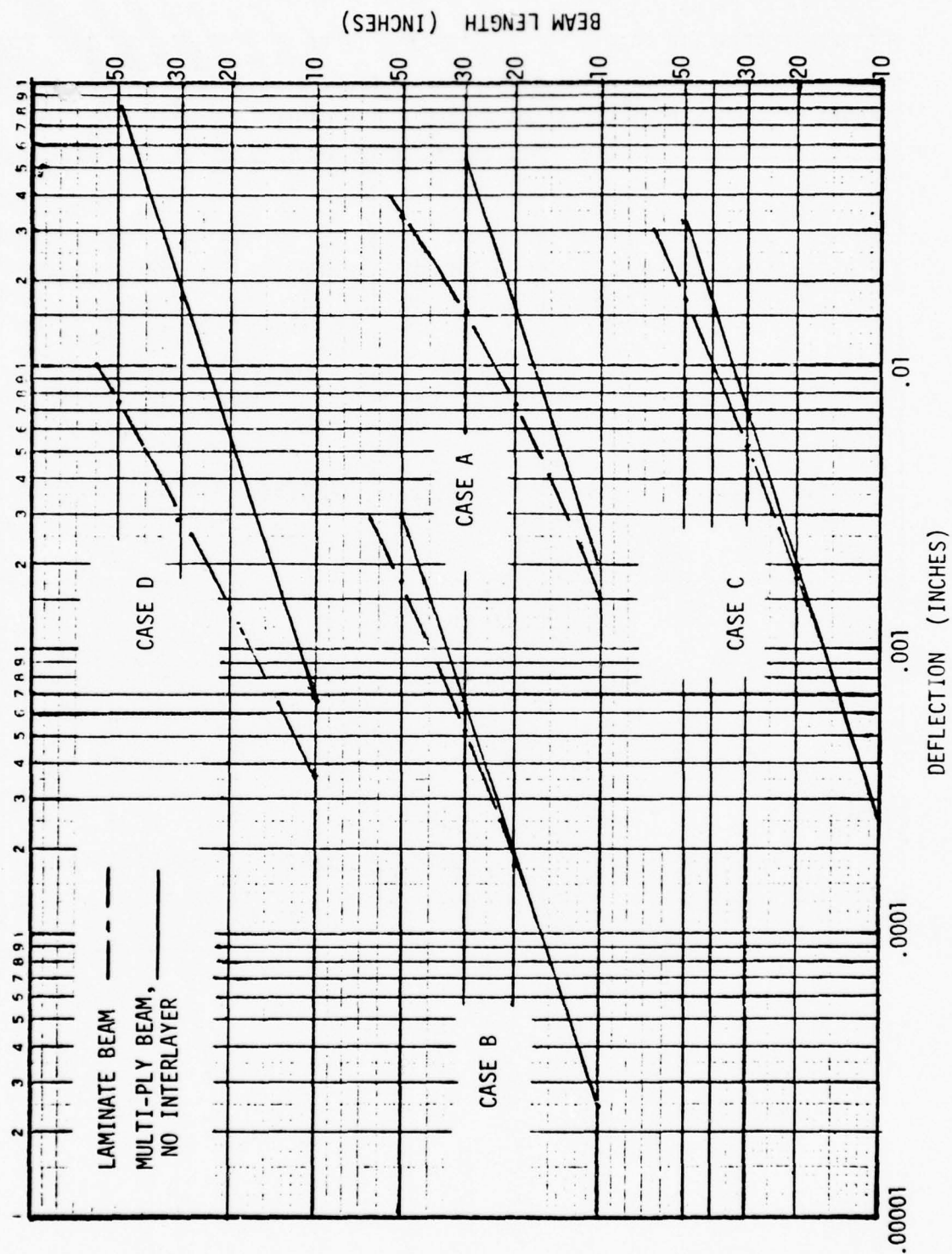


Figure 13. Deflection versus beam length.

where  $F$  is load,  $L$  is length and  $V$  is deflection. Effective stiffness varies with length. Table 4 is a comparison of effective stiffness, at various lengths for the four cases, to the stiffness if all the plies acted as individual beams. The reason why effective stiffness increases with length is that the interlayers are more effective in transferring shear between structural plies, as length increases. Thus, the behavior of the longer beams approaches monolithic.

Figure 14 is a plot of strain (inches per inch) versus thickness (inches) for Case A (4 ply polycarbonate) at the center of the beam with a one pound load applied at the center (beam length is ten inches). The values were calculated from the output data using the moment in the structural plies (MXJ) and the axial load in the structural plies (H).

Figure 15 shows a finite element model for a beam ten inches long with a cross section identical to Case A. The beam was analyzed using the computer aided Structural Design computer program\* to provide a comparison. The deflections agreed within one percent. This comparison is presented as verification of the validity of the basic assumptions for the enclosed derivations.

An actual test (Reference 1) was conducted to determine the deflection of the midpoint of a beam with a cross section similar to Case C, Figure 11. The beam length was 34.7 inches. The face-ply (ply No. 1) was "free floating". The deflection of the test beam was .00882 inch. The data output for the program is given in Appendix D and shows a deflection of 0.00913 inch which agrees within 3.5 percent. This comparison is presented as additional verification of the validity of the basic assumptions for the enclosed derivations.

\* Ref 9



TABLE 4. EFFECTIVE STIFFNESS (POUNDS-INCHES SQUARE)

LENGTH OF BEAM - (INCHES)	OUTPUT FROM PROGRAM				SINGLE ACTING BEAMS (NO INTERLAYER)
	10	20	30	50	
<u>CASE A</u>					
4-Ply Polycarbonate	3,397	5,683	9,238	19,212	2,600
<u>CASE B</u>					
2-Ply Glass	217,500	237,900	270,600	366,000	209,100
<u>CASE C</u>					
1-Ply Polycarbonate	20,700	23,450	27,500	37,900	19,720
<u>CASE D</u>					
2-Ply Polycarbonate	14,326	29,370	48,175	89,080	7,900

LENGTH = 10.0 INCH ———.  
 LENGTH = 50.0 INCH - - - -.

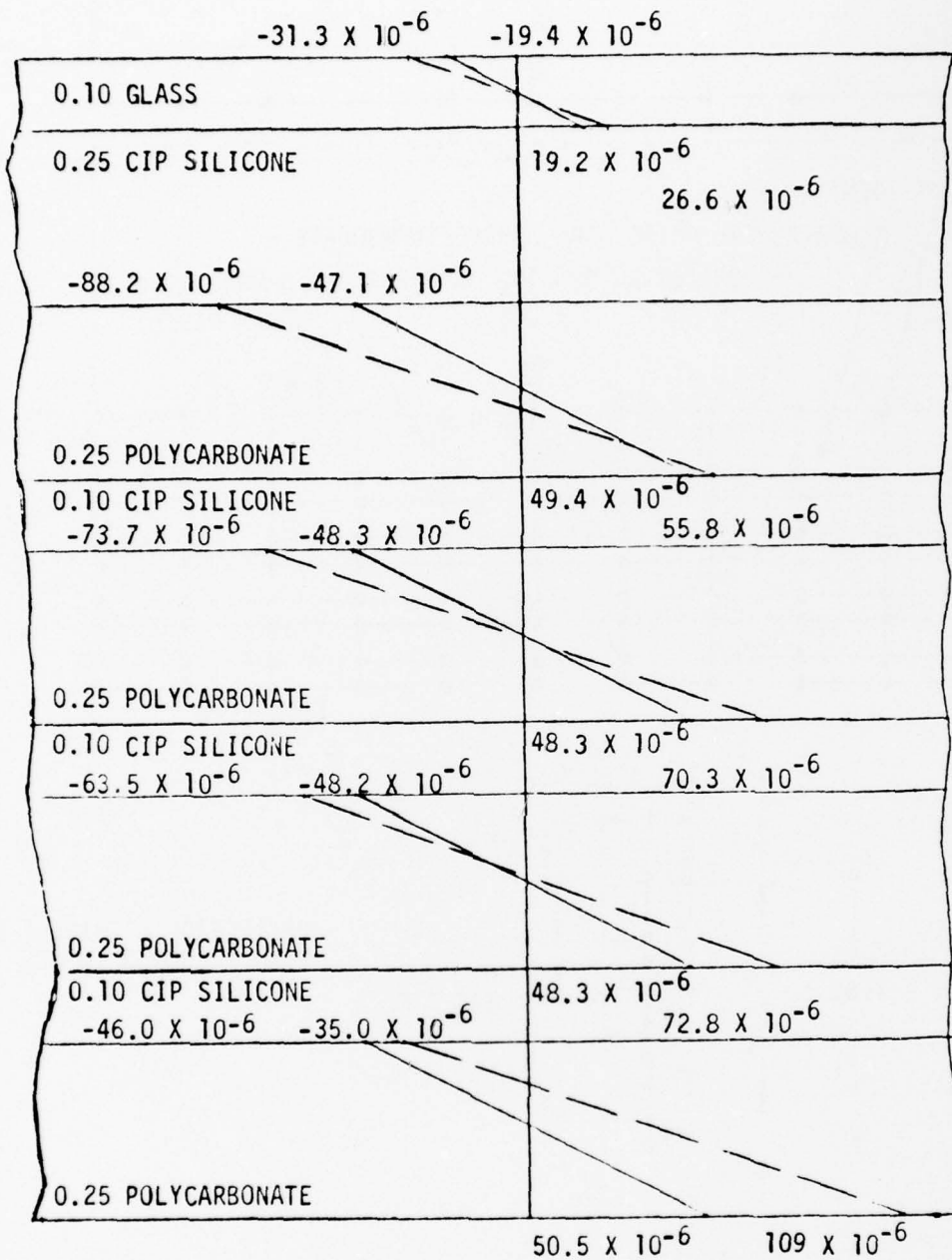


Figure 14. Strain (in/in) versus thickness for Case A at center of beam.

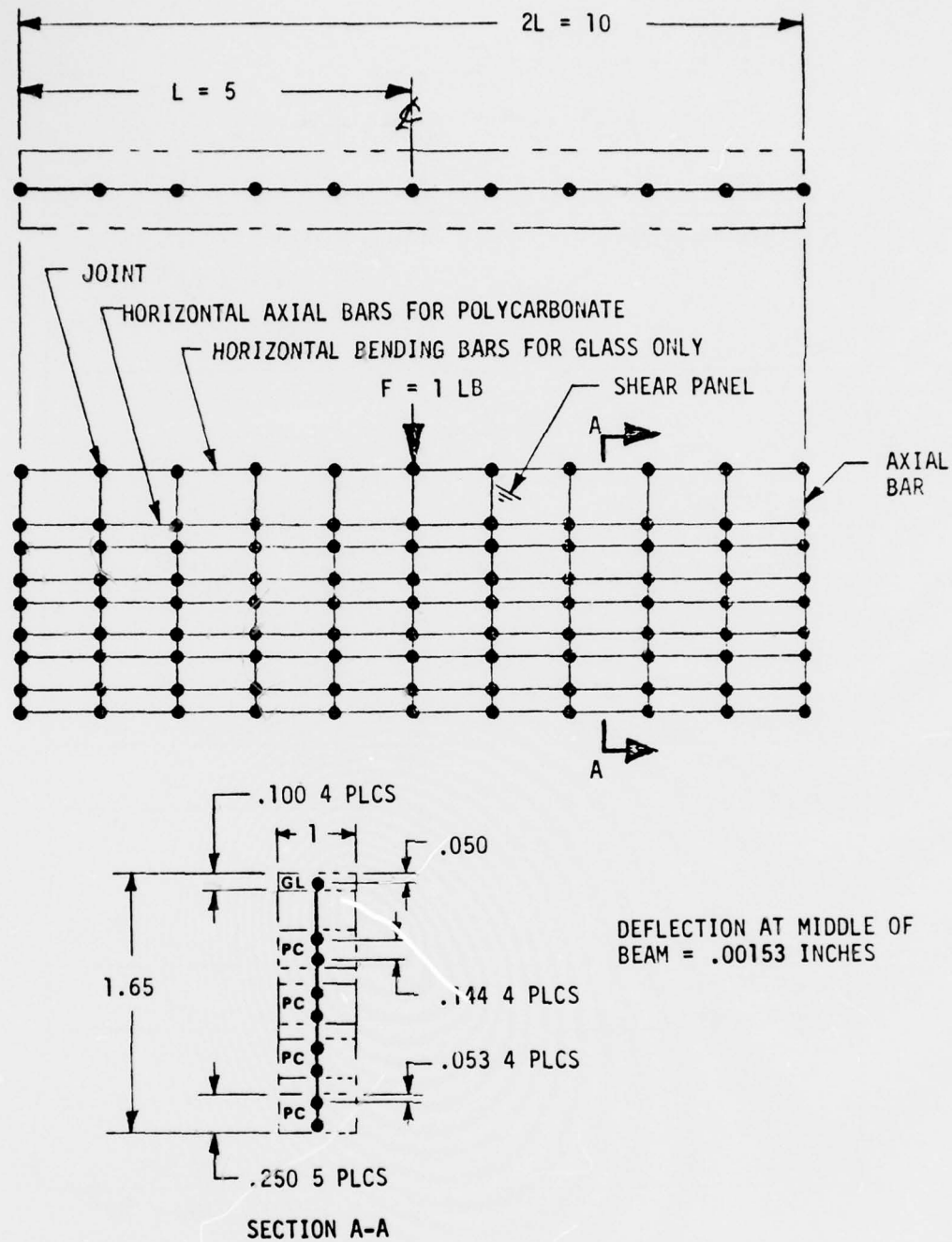


Figure 15. Finite element model of a fixed ended beam - Case A.

### SECTION III

#### COMPUTER PROGRAM USER'S MANUAL

A FORTRAN computer program has been coded which determines the internal loads, deflections, and relative stiffness of a laminated, fixed ended beam with one concentrated load at the center. The program is based on the equations and procedures embodied in the preceding sections of this report. A copy of the source coding is contained in Appendix B. Appendix C lists the intermediate matrices which were run for Case A. The output for four illustrative problems and one test beam is contained in Appendix D. The program was written in CDC FORTRAN Extended Version 4-0 for utilization of a CDC6600 computer.

Figure 16 shows a complete card deck ready to load for a particular computer installation. Certain obvious changes to the information shown on the cards, such as time charge numbers, programmer's name, and program identification, would have to be made to suit the requirements of another installation. A review of the source coding shown in Appendix B may reveal other minor changes that might be needed at another installation.

The complete card deck consists of essentially five categories: a job card, two control cards, a source deck, input data cards, and separating cards, called 7, 8, 9 Cards and 6, 7, 8, 9 Cards. Figure 17 shows the relationship of these components to the complete deck.

The job card, control cards, and separating cards may be obtained by entering the appropriate data on the data coding sheet, Figure 18, and submitting the sheet to procure the key-punched cards. The line entries are described as follows:

Line 1 - This line describes the job card which identifies the user and lists other pertinent identifying data.





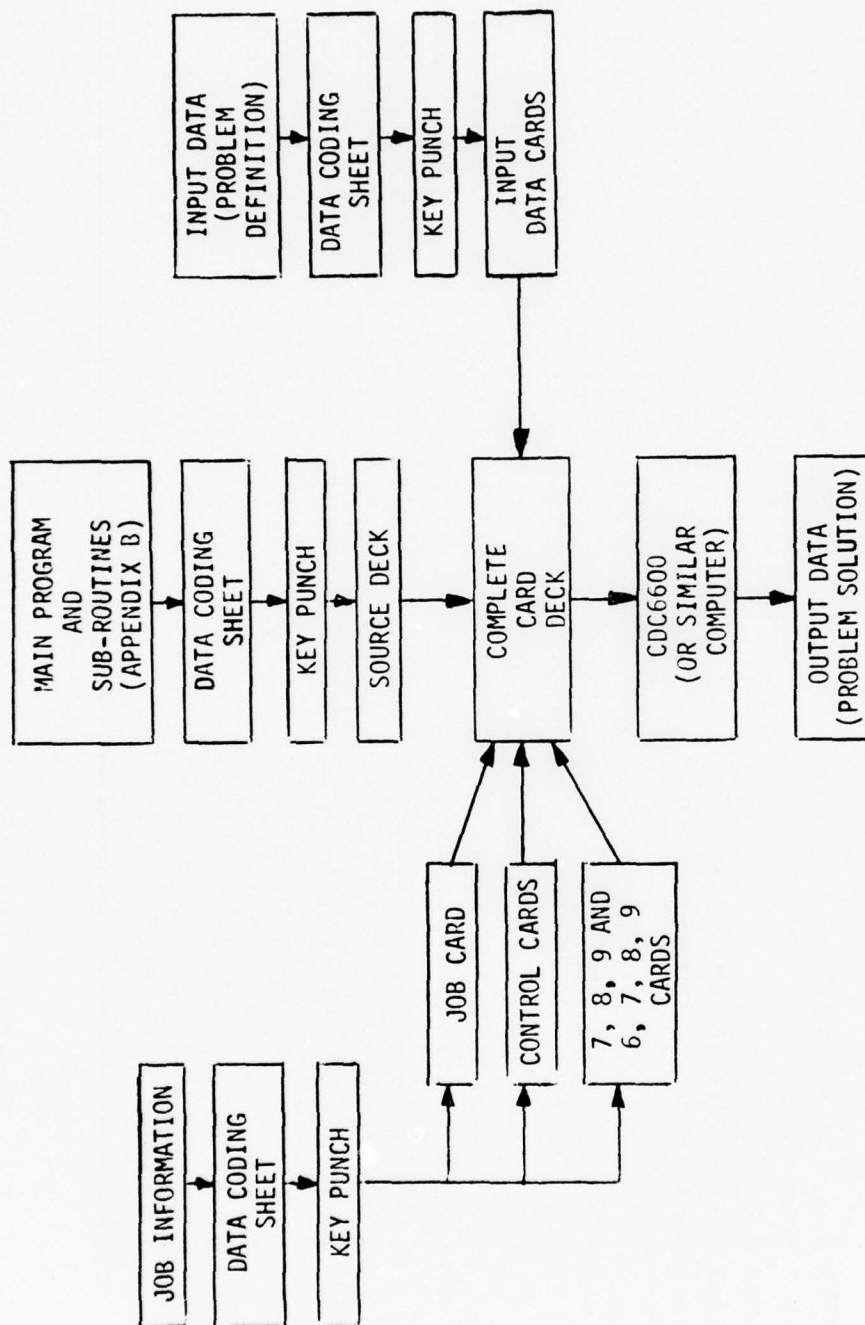


Figure 17. Card deck flow diagram.



Line 2 - This line refers to the FTN control card and provides access to library subroutines and compilation of the FORTRAN source deck.

Line 3 - This line refers to an LGO control card, which is a load and go card. One LGO card must be inserted for every set of data cards.

Line 4 - This line refers to a card that separates the source deck from the data cards and control cards. A minimum of two cards is required and an additional 7, 8, 9 Card must be added for each additional set of data cards.

Line 5 - This line refers to the last card in the deck, a 6, 7, 8, 9 Card.

The source deck is a set of approximately 1,700 cards and may be compiled from the main program and subroutines listed in Appendix B. This information may be entered on data coding sheets and submitted to key-punch to procure a source deck.

The input data card deck is a set of six cards and may be obtained by entering the appropriate information on the data coding sheet, Figure 19. The data sheet may be submitted to procure the key-punched cards shown in Figures 20 and 21 for illustrative Case A, Table 1. The program statements are shown for reference. A set of input data cards must be included for each problem to be solved. The line entries shown in Figure 19 for Case A are described as follows:

Line 1 - Columns 1 through 8 show the number of spanwise increments to the center line of the beam.

[illegible]



```

READ (5,10)NX,F,L,BB } PROGRAM STATEMENTS (REFERENCE ONLY)
10 FORMAT (I8,3F8.0) }

```

[illegible]

```
READ (5,20) (T(J),J = 1,9) } PROGRAM STATEMENTS (REFERENCE ONLY)
20 FORMAT (9F8.0)
```

[illegible]

```

READ (5,20) (E(J),J = 1,9) } PROGRAM STATEMENTS (REFERENCE ONLY)
20 FORMAT (9F8.0)

```

[illegible]

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DATA CARD 4. INPUT SHEAR MODULUS FOR EACH LAYER.

<pre> READ (5,20) (G(J),J = 1,9) } 20 FORMAT (9F8.0)           } </pre>	<pre> PROGRAM STATEMENTS (REFERENCE ONLY) </pre>
---	--

[illegible]

DATA CARD 5. INPUT DISTANCE FROM CENTER LINE OF BEAM TO X INCREMENT.  
(NUMBER OF VALUES MUST EQUAL NX ON CARD 1 AND LAST VALUE  
MUST BE EQUAL TO L ON CARD 1.) X(1) = 0

<pre> READ (5,20) (X(J),J = 2, NX1) } 20 FORMAT (9F8.0)                } </pre>	<pre> PROGRAM STATEMENTS (REFERENCE ONLY) </pre>
---	--

[illegible]

## DATA CARD 6. (CONTINUATION OF CARD 5.)

[illegible]

Figure 21. Sample data cards.

Line 1 - Columns 9 through 16 are the load in pounds.

Line 1 - Columns 17 through 24 are the length to the center line of the beam in inches.

Line 1 - Columns 25 through 32 are the width of the beam in inches.

Line 2 - Columns 1 through 8 are the thickness of the first ply in inches and the thickness of each of the remaining eight plies is shown in Columns 9 through 72.

Line 3 - Columns 1 through 8 are the modulus of elasticity (PSI) for Ply Number 1 and the modulus of elasticity for Ply Numbers 2 through 8 are shown in Columns 9 through 72.

Line 4 - Columns 1 through 8 are the shear modulus (PSI) for Ply Number 1 and the shear modulus for Ply Numbers 2 through 8 are shown in Columns 9 through 72.

Lines 5 and 6 - Columns 1 through 8 of Line 5 are the distance from the center of the beam to "X" increment in inches. The number of entries on Lines 5 and 6 correspond to the number of increments on Line 1, Columns 1 through 8. There are ten entries on Lines 5 and 6.

Lines 7 through 12 - These entries are the appropriate computer symbols corresponding to the entries on Lines 1 through 6 above and are for reference only.

Data cards must meet the following requirements:

- ° Each card field is eight (8) card columns wide.

- ° There cannot be any zeros or blank fields imbedded in the data (i.e., If a layer is to be dropped from the problem, its thickness and other parameters must be input to the problem as relatively small numbers to cause its effective elimination.).
- ° Numbers must be right justified in their fields.
- ° A trailing decimal point may be dropped.

## SECTION IV CONCLUSIONS

The analysis and computer program presented in this report were developed to provide a tool for evaluating laminated combinations of transparent materials during initial design of windshields and windows. Historically, formulas and theories from engineering handbooks have been used for this purpose, but these methods are highly approximate.

The laminated beam analyzed in the present approach is considered to represent a strip cut from a typical aircraft windshield transparency. The primary characteristic of such a laminated beam is the relative ease of structural plies to slide past each other because of the softness of interlayer materials. Equilibrium and compatibility equations are written for the beam, based upon assumptions that preserve the important features of laminated beam behavior. The resulting set of differential equations is solved exactly.

The computer code presented can expedite the application of the theory to practical problems. Utilizing this program, the deflections for a series of typical laminated beams were examined, and were compared to the results of a proven finite element computer program. The two sets of computed values agreed within one percent. Data obtained from another beam, tested as noted in Reference 1, were compared to calculated results provided by the present method. The outer ply of the test beam was "free floating" at the ends, rather than fixed, as assumed in the analysis. To simulate this condition, a negligible value of Young's modulus was assigned to the outer ply. The calculated deflection was 3.5 percent greater than the measured value. This agreement is considered reasonably close, in view of the approximate method used to account for the "free floating" outer ply.

The applications of the computer program described in Section II under the heading "Illustrative Examples" were run on the CDC6600

computer, employing 21,000 decimal words of core. Each case required 0.879 seconds CPU time and 7.588 seconds I/O time. The I/O time includes compilation of the entire program.



## SECTION V

### RECOMMENDATIONS

The new computer program is recommended as a tool for use in the early stages of aircraft windshield design as a means of screening laminate configurations. As an example, a strip of a certain width can be cut out of a transparency being studied. This strip can be flattened to form a laminated beam having pinned or fixed ends as appropriate. The width of the strip can be selected so that the resulting beam, supported only at the ends, roughly represents the transparency supported on all edges. A load can be applied at the center to represent a bird impact. The number of plies, structural ply and interlayer materials, and ply thicknesses can be varied, so that established stress and/or displacement constraints are met. The appropriate constraints are that ultimate stresses and maximum allowable displacements are not exceeded. Weights per square foot of transparency surface can then be calculated and compared. The laminates showing the most promise on this basis can be selected for further study.

Another use for the present code is to provide an effective bending stiffness for a laminate. This effective stiffness can be selected as the EI value of a monolithic beam necessary to give the same maximum deflection as a beam composed of the given laminate under the same load. The computer program outputs such a value. This value can then be employed as a means of developing a finite element model of the laminated transparency and supporting structure, in which the laminate is replaced by a single layer, as a means of reducing engineering, computing, and elapsed time to perform either a static or a dynamic analysis. Since effective stiffness depends upon beam length, the user should estimate the half length of the wave produced in the transparency when impact occurs. Such an estimate can be made on the basis of past experience, including previous studies employing the Bird Impact Math Model (Reference 6) or test results. This half wave length can then be taken as the length of the simply supported beam for which an effective EI

value can be calculated. An approach of this kind is probably most useful for calculating transparency displacements and loads on edge attachments and supporting structure.

The feasibility of extending the method to cover other loadings and boundary conditions should be studied. For example, an option applicable to an end condition in which the layers are prevented from sliding relative to each other, as by a bolt, although the end tangent can rotate, might be useful. A beam uniformly loaded over part of its length is a candidate case. The feasibility of applying the approach to dynamic problems also should be studied, and the possibility of eliminating the assumption that transverse strains are negligible should be considered. Eliminating this assumption would mean that the method would provide reasonably reliable estimates of transverse stresses, and perhaps a means of forecasting delamination. Any of these improvements would increase the realism of the analytical results. Consequently, the usefulness of the approach as a design tool would be enhanced.

If any of these improvements were accomplished, they could be verified by comparison with results obtained by finite element analysis.

APPENDIX A  
PROPERTIES OF THE EIGENVALUES  
AND EIGENVECTORS

The matrix differential equation (Equation 42) can be written in the following form by rearranging the rows of  $Y$  :

$$\begin{Bmatrix} dY_a/dx \\ dY_b/dx \end{Bmatrix} = \begin{bmatrix} 0 & a_b \\ b_a & 0 \end{bmatrix} \begin{Bmatrix} Y_a \\ Y_b \end{Bmatrix} \quad (A.1)$$

where

$$b_a = \begin{bmatrix} a_{2,1} & a_{2,3} & 0 \\ 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix} \quad Y_a = \begin{Bmatrix} u \\ \theta \\ \phi \end{Bmatrix} \quad (A.2)$$

$$a_b = \begin{bmatrix} a_{1,2} & 0 & 0 \\ 0 & 1 & 0 \\ a_{5,2} & a_{5,4} & 0 \end{bmatrix} \quad Y_b = \begin{Bmatrix} H \\ \beta \\ v \end{Bmatrix} \quad (A.3)$$

The corresponding characteristic equation is

$$\begin{bmatrix} -\lambda I & a_b \\ b_a & -\lambda I \end{bmatrix} \begin{Bmatrix} G_a \\ G_b \end{Bmatrix} = 0 \quad (A.4)$$

where

$$G_a = \begin{Bmatrix} G_u \\ G_\theta \\ G_\phi \end{Bmatrix} \quad G_b = \begin{Bmatrix} G_H \\ G_\beta \\ G_v \end{Bmatrix} \quad (A.5)$$

$$\therefore \begin{cases} -\lambda G_a + a_b G_b = 0 \\ b_a G_a - \lambda G_b = 0 \end{cases} \quad (A.6)$$

$$(A.7)$$

Changing the signs of  $\lambda$  and  $G_b$  leaves Equations A.5 and A.6 unchanged.

Therefore if  $\lambda$  and  $\begin{Bmatrix} G_a \\ G_b \end{Bmatrix}$  are an eigenvalue and an eigenvector of Equation A.4, then  $-\lambda$  and  $\begin{Bmatrix} G_a \\ -G_b \end{Bmatrix}$  are also an eigenvalue and an eigenvector.

APPENDIX B

SOURCE CODING (MAIN PROGRAM AND SUB-ROUTINES)



```

10  PROGRAM MAIN(INPUT=512,TAPE5=INPUT,OUTPUT=512,TAPE6=OUTPUT)
20  EQUIVALENCE (AL,LA),(TEMP(1,1),WORK(1000))
30  REAL L,KA,KETA,KI,LAMBL,MXJ
40  COMPLEX EIGVAL(14),EIGVEC(14,14)
50  DIMENSION WORK(3000),T(9),E(9),G(9),X(100),KA(5,5)
60  KETA(4,4),XKI(5),TBAR(4),DELTAQ(5,4),A12(5,5),A21(5,5)
70  A23(5),A52(5),A54(1),A(14,14),FLDL(4,4),Q(5,4),U1(5)
80  U2(5),B(14,14),HL(14,4),LAMBL(4),PSI(14),FLD(4,4)
90  FGD(4,4),ANS(14),QXJ(4),VQ(5,4),VXJ(5),MXJ(5)
100 TEMP(14,6)
110 S22(4,4),S12(4),XT(5),W(5)
120
130  NX = NUMBER OF X'S.
140  F = FORCE.
150  L = LENGTH.
160  BB = WIDTH.
170  WRITE(6,935)
180  READ(5,10) NX,F,L,BB
190  FORMAT(18,3F8.0)
200
210  READ T,E,G.
220  READ(5,20) (T(J),J=1,9)
230  READ(5,20) (E(J),J=1,9)
240  READ(5,20) (G(J),J=1,9)
250  X(1) = 0.0
260  NX1 = NX+1
270  READ X.
280  READ(5,20) (X(J),J=2,NX1)
290  FORMAT(9F8.0)
300  WRITE(6,1) NX,F,L,BB
310  WRITE(6,2) (T(J),J=1,9)
320  WRITE(6,3) (E(J),J=1,9)
330  WRITE(6,4) (G(J),J=1,9)
340  WRITE(6,5) (X(J),J=1,NX1)
350  1 FORMAT(11H NX,F,L,BB,I5,1P3E16.6)
360  2 FORMAT(3H T,1P5E16.6/3H,1P4E16.6)
370  3 FORMAT(3H E,1P5E16.6/3H,1P4E16.6)
380  4 FORMAT(3H G,1P5E16.6/3H,1P4E16.6)

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5 FORMAT(3H X ,1P5E16.6/3H ,1P6E16.6)
C
  FORM VQ
  DO 90 I=1,5
  DO 90 J=1,4
  90 VQ(I,J) = 0.0
  DO 95 I=1,4
  VQ(I,I) = 0.5*T(2*I-1)
  95 VQ(I+1,I) = 0.5*T(2*I+1)
C
  FORM KA
  DO 100 I=1,5
  DO 100 J=1,5
  100 KA(I,J) = 0.0
  DO 150 I=1,5
  150 KA(I,I) = E(2*I-1)*BB*T(2*I-1)
C
  FORM KETA
  DO 160 I=1,4
  DO 160 J=1,4
  160 KETA(I,J) = 0.0
  DO 170 I=1,4
  170 KETA(I,I) = G(2*I)*BB/T(2*I)
C
  FORM KI,XKI
  KI = 0.0
  DO 180 I=1,5
  XKI(I) = E(2*I-1)*BB*T(2*I-1)**3/12.0
  180 KI = KI+XKI(I)
C
  FORM TBAR
  DO 190 I=1,4
  190 TBAR(I) = 0.5*T(2*I-1)+T(2*I)+0.5*T(2*I+1)
C
  FORM DELTAQ
  DO 200 I=1,5
  DO 200 J=1,4
  200 DELTAQ(I,J) = 0.0
  DO 210 I=1,4
  DELTAQ(I,I) = -1.0
  210 DELTAQ(I+1,I) = 1.0
C
  FORM A12

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C
220 IF(I.EQ. J) A12(I,J) = 1.0/KA(I,J)
    FORM A21
CALL MULT(DELTAQ,5,4,KETA,4,4,WORK)
CALL TMULT(WORK,5,4,DELTAQ,5,4,A21)
    FORM A23
C
DO 230 I=1,20
230 WORK(I) = -WORK(I)
    CALL MULT(WORK,5,4,TBAR,4,1,A23)
    FORM A52
C
CALL LTMULT(A23,5,1,A12,5,5,A52)
DO 240 I=1,5
240 A52(I) = A52(I)/KI
    FORM A54
C
CALL LTMULT(TBAR,4,1,KETA,4,4,WORK)
CALL MULT(WORK,1,4,TBAR,4,1,A54)
A54(1) = A54(1)/KI
    FORM A
C
DO 250 I=1,14
DO 250 J=1,14
250 A(I,J) = 0.0
DO 260 I=1,5
DO 260 J=1,5
A(I+5,J) = A21(I,J)
260 A(I,J+5) = A12(I,J)
DO 270 J=1,5
270 A(14,J+5) = A52(J)
DO 280 I=1,5
280 A(I+5,12) = A23(I)
A(14,13) = A54(1)
DO 290 I=11,13
290 A(I,I+1) = 1.0
EPSILN = 0.00001
DO 299 J=1,14

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1090 WRITE(6,298) (A(I,J),I=1,14)
1100 FORMAT(3H A ,1P7E16.6/1H ,1P7E16.6)
1110 299 CONTINUE
1120 IND = 1
1130 CALL EIGEN ROUTINE.
1140 CALL RGEIG(14,14,A,IND,EIGVAL,EIGVEC,WORK)
1150 DO 310 I=1,14
1160 310 WORK(I) = REAL(EIGVAL(I))
1170 WRITE(6,311) (WORK(I),I=1,14)
1180 311 FORMAT(13H EIGENVALUES ,1P7E16.6/1H ,1P7E16.6)
1190 C TEST EIGENVALUES,
1200 DO 320 I=1,14
1210 COM = AIMAG(EIGVAL(I))
1220 IF(ABS(COM) .LE. EPSILN) GO TO 320
1230 WRITE(6,315) COM,I
1240 315 FORMAT(19H COMPLEX EIGENVALUE,1PE13.6,I10)
1250 GO TO 9999
1260 320 CONTINUE
1270 340 KK = 1
1280 KOUNT = 0
1290 NNEG = 0
1300 NPOS = 0
1310 DO 371 I=1,14
1320 IF(WORK(I) .GT. EPSILN) NPOS=NPOS+1
1330 IF(WORK(I) .LT. -EPSILN) NNEG=NNEG+1
1340 IF(ABS(WORK(I)) .GT. EPSILN) GO TO 350
1350 KOUNT = KOUNT+1
1360 GO TO 371
1370 350 ITEST = 0
1380 DO 360 J=1,14
1390 IF(ABS(WORK(I)+WORK(J)) .GT. EPSILN) GO TO 360
1400 ITEST = 1
1410 GO TO 361
1420 360 CONTINUE
1430 361 IF(ITEST .EQ. 1) GO TO 363
1440 WRITE(6,362)

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362 FORMAT(22H UNMATCHED EIGENVALUES)
GO TO 9999
363 IF(WORK(1) .GT. 0) GO TO 371
IF(KK .LE. 4) GO TO 365
WRITE(6,364)
364 FORMAT(9H KK GT 4)
GO TO 9999
STORE DESIRED EIGENVALUES.
365 LAMBL(KK) = WORK(I)
DO 369 K=1,14
COM = AIMAG(EIGVEC(K,I))
IF(COM .LE. EPSILN) GO TO 369
WRITE(6,368) COM,I,K
368 FORMAT(16H COMPLEX VECTOR ,1PE13.6,2I5)
GO TO 9999
369 CONTINUE
STORE DESIRED EIGENVECTORS,
DO 370 K=1,14
370 HL(K,KK) = REAL(EIGVEC(K,I))
KK = KK+1
371 CONTINUE
IF((KOUNT .EQ. 6) .AND. (NPOS .EQ. 4) .AND. (NNEG .EQ. 4))
160 TO 400
WRITE(6,375) KOUNT,NPOS,NNEG
375 FORMAT(17H KOUNT,NPOS,NNEG ,3I5)
GO TO 9999
400 CONTINUE
FORM FLDL
DO 408 I=1,4
DO 408 J=1,4
408 FLDL(I,J) = 0.0
DO 409 I=1,4
409 FLDL(I,I) = EXP(LAMBL(I)*L)
FORM Q
DO 410 I=1,5
DO 410 J=1,4

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410 Q(I,J) = 0.0
DO 420 I=1,4
DO 420 J=1,4
420 Q(I,J) = -1.0
FORM U1 AND U2
DO 425 I = 1,5
U2(I) = 0.0
XT(I) = 0.0
DO 425 J = 1,5
425 XT(I) = XT(I) + KA(I,J)
CALL MULT ( KA,5,5,Q,5,4,WORK(1) )
CALL MULT ( WORK(1),5,4,TBAR,4,1,W )
DO 426 I = 1,4
426 S12(I) = A21(1,I+1)
CALL MULT ( XT(2),4,1,S12,1,4,S22 )
XT11 = 1.0 / XT(1)
DO 427 I = 1,4
U2(I+1) = W(I+1) - XT11 * XT(I+1) * W(1)
DO 427 J = 1,4
427 S22(I,J) = A21(I+1,J+1) - XT11 * S22(I,J)
CALL SLEQ ( S22,U2(2),4,IER )
CALL MULT ( S12,1,4,U2(2),4,1,ALPHA )
ALPHA = XT11 * ( ALPHA - W(1) )
CALL MULT ( Q,5,4,TBAR,4,1,U1 )
DO 430 I = 1,5
430 U1(I) = U1(I) + ALPHA
FORM B
DO 435 I=1,14
DO 435 J=1,14
435 B(I,J) = 0.0
DO 440 I=1,5
DO 440 J=1,4
B(I,J) = HL(I,J)
440 B(I+8,J+4) = HL(I,J)
DO 450 J=1,4
450 B(6,J) = HL(11,J)

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DO 460 J=1,4
  B(7,J) = HL(12,J)
460 B(14,J+4) = HL(12,J)
DO 470 J=1,4
  B(8,J) = HL(14,J)
470 B(8,J) = HL(14,J)
DO 480 I=1,5
  B(1,9) = 1.0
  B(1+8,9) = 1.0
480 B(1+8,10) = L
  B(6,11) = 1.0
  B(7,12) = 1.0
  B(8,14) = 1.0
  B(14,12) = 1.0
  B(14,13) = L
  B(14,14) = L*L/2.0
DO 490 I=1,5
  B(1,12) = U1(I)
  B(1+8,12) = U1(I)
  B(1+8,13) = U1(I)*L
  B(1,14) = U2(I)
490 B(1+8,14) = 0.5*U1(I)*L*U2(I)
DO 500 J=1,4
DO 500 I=1,5
  WORK(I+5*(J-1)) = HL(I,J)
  CALL MULT(WORK,5,4,FLDL,4,4,WORK(21))
DO 510 J=1,4
DO 510 I=1,5
  IJ = I+20+5*(J-1)
  B(1+8,J) = WORK(IJ)
510 B(I,J+4) = WORK(IJ)
DO 520 J=1,4
520 WORK(J) = HL(12,J)
  CALL MULT(WORK,1,4,FLDL,4,4,WORK(5))
DO 530 J=1,4
  B(14,J) = WORK(J+4)
530 B(7,J+4) = WORK(J+4)

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```

2530      DO 540 J=1,4
2540      540 WORK(J) = -HL(11,J)
2550      CALL MULT(WORK,1,4,FLDL,4,4,WORK(5))
2560      DO 550 J=1,4
2570      550 B(6,J+4) = WORK(J+4)
2580      DO 560 J=1,4
2590      560 WORK(J) = HL(14,J)
2600      CALL MULT(WORK,1,4,FLDL,4,4,WORK(5))
2610      DO 570 J=1,4
2620      570 B(8,J+4) = WORK(J+4)
2630      P = F/2.0
2640      FORM PSI
2650      DO 580 J=1,14
2660      580 PSI(J) = 0.0
2670      PSI(8) = -P/KI
2680      CALL SOLUTION OF SIMULTANEOUS LINEAR EQUATIONS ROUTINE.
2690      CALL SLEQ(B,PSI,14,IER)
2700      LOOP OVER ALL X.
2710      DO 1000 JELT=1,NX1
2720      FORM FLD,FGD
2730      DO 700 I=1,4
2740      DO 700 J=1,4
2750      FLD(I,J) = 0.0
2760      FGD(I,J) = 0.0
2770      IF(I .NE. J) GO TO 700
2780      FLD(I,J) = EXP(LAMBL(I)*X(JELT))
2790      FGD(I,J) = EXP(LAMBL(I)*(L-X(JELT)))
2800      700 CONTINUE
2810      FORM ANS
2820      DO 710 J=1,4
2830      710 WORK(J) = PSI(J)
2840      CALL MULT(FLD,4,4,WORK,4,1,WORK(5))
2850      CALL MULT(HL,14,4,WORK(5),4,1,ANS)
2860      DO 720 J=1,4
2870      720 WORK(J) = PSI(J+4)
2880      CALL MULT(FGD,4,4,WORK,4,1,WORK(5))

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2890      DO 730 I=6,11
2900      DO 730 J=1,4
2910      HL(I,J) = -HL(I,J)
2920      DO 740 J=1,4
2930      HL(13,J) = -HL(13,J)
2940      CALL MULT(HL,14,4,WORK(5),4,1,WORK(10))
2950      DO 750 I=1,14
2960      ANS(I) = ANS(I)+WORK(I+9)
2970      DO 760 I=6,11
2980      DO 760 J=1,4
2990      HL(I,J) = -HL(I,J)
3000      DO 770 J=1,4
3010      HL(13,J) = -HL(13,J)
3020      DO 780 I=1,14
3030      DO 780 J=1,6
3040      TEMP(I,J) = 0.0
3050      DO 790 I=1,5
3060      TEMP(I,1) = 1.0
3070      TEMP(I,2) = X(JELT)
3080      TEMP(I,4) = U1(I)
3090      TEMP(I,5) = U1(I)*X(JELT)
3100      TEMP(I,6) = 0.5*TEMP(I,5)*X(JELT)+U2(I)
3110      DO 800 I=1,5
3120      TEMP(I+5,2) = KA(I,1)
3130      CALL MULT(KA,5,5,U1,5,1,A23)
3140      DO 810 I=1,5
3150      AB = A23(I)
3160      TEMP(I+5,5) = AB
3170      TEMP(I+5,6) = AB*X(JELT)
3180      DO 820 I=1,14
3190      TEMP(I,1-8) = 1.0
3200      DO 830 I=1,13
3210      TEMP(I,1-7) = X(JELT)
3220      TEMP(11,5) = X(JELT)*X(JELT)*0.5
3230      TEMP(12,6) = TEMP(11,5)
3240      TEMP(11,6) = TEMP(11,5)*X(JELT)/3.0

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      DO 840 I=1,6
      840 WORK(I) = PSI(I+8)
      CALL MULT(TEMP,14,6,WORK,6,1,WORK(10))
      DO 850 I=1,14
      850 ANS(I) = ANS(I)+WORK(I+9)
      C
      FORM QXJ
      DO 860 I=1,4
      860 WORK(I) = TBAR(I)*ANS(12)
      DO 870 I=1,5
      870 A23(I) = ANS(I)
      CALL LTMULT(DELTAQ,5,4,A23,5,1,WORK(10))
      DO 880 I=1,4
      880 WORK(I) = WORK(I)-WORK(I+9)
      CALL MULT(KETA,4,4,WORK,4,1,QXJ)
      C
      FORM VXJ
      CALL MULT(VQ,5,4,QXJ,4,1,A23)
      DO 910 I=1,5
      910 A52(I) = XKI(I)*ANS(14)
      DO 920 I=1,5
      920 VXJ(I) = A23(I)-A52(I)
      C
      FORM MXJ
      DO 930 I=1,5
      930 MXJ(I) = XKI(I)*ANS(13)
      J1 = JELT-1
      C
      OUTPUT DESIRED MATRICES.
      935 FORMAT(1H1)
      WRITE(6,940) J1
      940 FORMAT(1H /3H X(,I5,1H))
      WRITE(6,950) (ANS(I),I=1,5)
      950 FORMAT(1H /3H U ,1P5E16.6)
      WRITE(6,960) (ANS(I),I=6,10)
      960 FORMAT(1H /3H H ,1P5E16.6)
      WRITE(6,970) (ANS(I),I=11,14)
      970 FORMAT(1H /5H VEE ,1PE16.6,7H THETA ,1PE16.6,5H P
      1H1 ,1PE16.6)
      WRITE(6,980) (QXJ(I),I=1,4)

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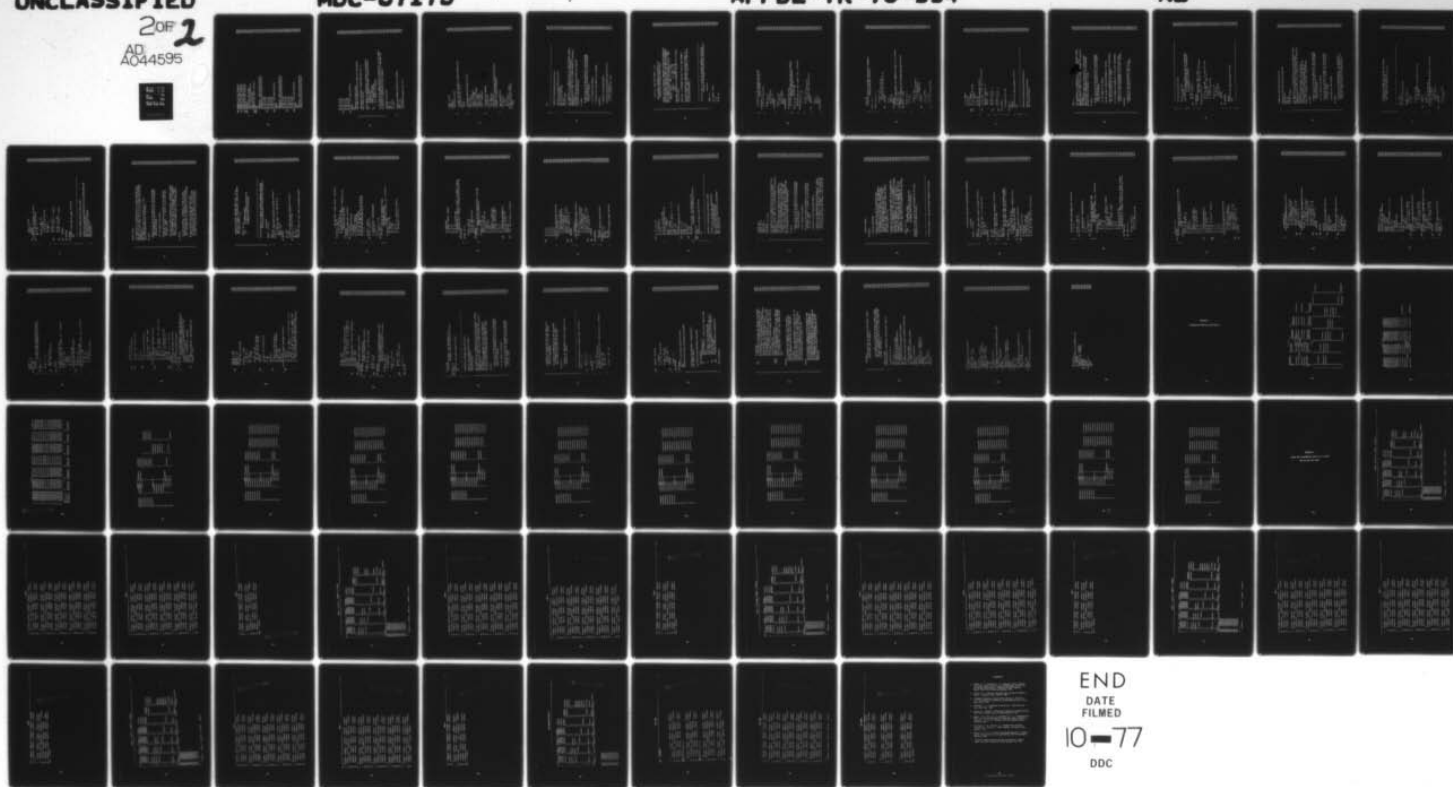
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980 FORMAT(1H /5H QXJ ,1P4E16.6)
981 WRITE(6,990) (VXJ(I),I=1,5)
990 FORMAT(1H /5H VXJ ,1P5E16.6)
991 WRITE(6,995) (MXJ(I),I=1,5)
995 FORMAT(1H /5H MXJ ,1P5E16.6)
1000 CONTINUE
      FORM EI(EFF) AND OUTPUT.
      EIEFF = P*L**3/(12.0*ANS(11))
      WRITE(6,1100) EIEFF
1100 FORMAT(1H /9H EI(EFF) ,1PE16.6)
9999 STOP
      END
      SUBROUTINE MULT(A,N1,M1,B,N2,M2,C)
      DIMENSION A(N1,M1),B(N2,M2),C(N1,M2)
      DO 100 I=1,N1
      DO 100 J=1,M2
      100 C(I,J) = 0.0
      DO 200 I=1,N1
      DO 200 J=1,M2
      DO 200 K=1,M1
      200 C(I,J) = C(I,J)+A(I,K)*B(K,J)
      RETURN
      END
      SUBROUTINE TMULT(A,N1,M1,B,N2,M2,C)
      DIMENSION A(N1,M1),B(N2,M2),C(N1,N2)
      DO 100 I=1,N1
      DO 100 J=1,N2
      100 C(I,J) = 0.0
      DO 200 I=1,N1
      DO 200 J=1,N2
      DO 200 K=1,M1
      200 C(I,J) = C(I,J)+A(I,K)*B(K,J)
      RETURN
      END
      SUBROUTINE LTMULT(A,N1,M1,B,N2,M2,C)
      DIMENSION A(N1,M1),B(N2,M2),C(M1,M2)

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3970 DO 100 I=1,M1
3980 DO 100 J=1,M2
3990 100 C(I,J) = 0.0
4000 DO 200 I=1,M1
4010 DO 200 J=1,M2
4020 DO 200 K=1,N1
4030 200 C(I,J) = C(I,J)+A(K,I)*B(K,J)
4040 RETURN
4050 END
4060 SUBROUTINE SLEQ(A,B,N,IER)
4070 SLEQ FROM THE COMPUTER CENTER LIBRARY OF 6600 ROUTINES
4080
4090 SIMULTANEOUS SOLUTION OF LINEAR EQUATIONS, AX = B.
4100 CALL SLEQ(A,B,N,IER)
4110
4120 A(N,N) MATRIX OF COEFFICIENTS, DESTROYED IN COMPUTATION.
4130 B(N) VECTOR OF ORIGINAL CONSTANTS, SOLUTION VECTOR X
4140 RETURNED HERE.
4150 N NUMBER OF EQUATIONS.
4160 IER IF 0, NORMAL SOLUTION
4170 IF 1, MATRIX A IS SINGULAR
4180
4190 IF NO PIVOT CAN BE FOUND EXCEEDING A TOLERANCE OF 0.0, THE MATRIX
4200 IS CONSIDERED SINGULAR AND IER IS SET TO 1. THIS TOLERANCE CAN
4210 BE MODIFIED BY REPLACING THE FIRST STATEMENT.
4220
4230 DIMENSION A(N,N),B(N)
4240 TOL = 0.
4250 IER = 0
4260 DO 8 J=1,N
4270 J1 = J+1
4280
4290 SEARCH FOR MAXIMUM COEFFICIENT IN COLUMN J.
4300 PIVOT = 0.
4310 DO 2 I=J,N
4320 IF(ABS(PIVOT)-ABS(A(I,J))) 1,2,2

```

	1 PIVOT = A(I,J)	4330
	IMAX = I	4340
	2 CONTINUE	4350
	WRITE(6,100) PIVOT	4360
C		4370
	IF PIVOT LESS THAN TOLERANCE (SINGULAR MATRIX) EXIT.	4380
C	IF (ABS(PIVOT)-TOL) 3,3,4	4390
	3 IER = 1	4400
	RETURN	4410
		4420
C	INTERCHANGE ROWS AND DIVIDE BY PIVOT.	4430
C	DO 5 K=J,N	4440
	TEMP = A(J,K)	4450
	A(J,K) = A(IMAX,K)	4460
	A(IMAX,K) = TEMP	4470
	5 A(J,K) = A(J,K)/PIVOT	4480
	TEMP = B(IMAX)	4490
	B(IMAX) = B(J)	4500
	B(J) = TEMP/PIVOT	4510
		4520
C	ELIMINATE NEXT VARIABLE	4530
C	IF (J-N) 6,9,6	4540
	6 J1 = J+1	4550
	DO 8 IROW=J1,N	4560
	DO 7 JCOL=J1,N	4570
	7 A(IROW,JCOL) = A(IROW,JCOL)-A(IROW,J)*A(J,JCOL)	4580
	8 B(IROW) = B(IROW)-B(J)*A(IROW,J)	4590
		4600
		4610
	BACK SUBSTITUTION	4620
C	DO 10 K=2,N	4630
C	I = N+1-K	4640
	I1 = I+1	4650
	DO 10 J=I1,N	4660
	10 B(I) = B(I)-A(I,J)*B(J)	4670
	100 FORMAT(7H PIVOT ,1PE16.6)	4680
	RETURN	

C	END	4690
C	-----	4700
C		4710
C		4720
C	SUBROUTINE BALANC(NM,N,A,LOW,IGH,SCALE)	4730
		4740
	INTEGER I,J,K,L,M,N,JJ,NM,IGH,LOW,IEXC	4750
	REAL A(NM,N),SCALE(N)	4760
	REAL C,F,G,R,S,B2,RADIX	4770
	REAL ABS	4780
C	LOGICAL NOCONV	4790
C		4800
C	THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE BALANCE,	4810
C	NUM. MATH. 13, 293-304(1969) BY PARLETT AND REINSCH.	4820
C	HANDBOOK FOR AUTO. COMP., VOL.11-LINEAR ALGEBRA, 315-326(1971).	4830
C	(REFERENCE 7)	4840
C	THIS SUBROUTINE BALANCES A REAL MATRIX AND ISOLATES	4850
C	EIGENVALUES WHENEVER POSSIBLE.	4860
C		4870
C	ON INPUT-	4880
C		4890
C	NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL	4900
C	ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM	4910
C	DIMENSION STATEMENT.	4920
C		4930
C	N IS THE ORDER OF THE MATRIX.	4940
C		4950
C	A CONTAINS THE INPUT MATRIX TO BE BALANCED.	4960
C		4970
C	ON OUTPUT-	4980
C		4990
C	A CONTAINS THE BALANCED MATRIX.	5000
C		5010
C	LOW AND IGH ARE TWO INTEGERS SUCH THAT A(I,J)	5020
C	IS EQUAL TO ZERO IF	5030
C	(1) I IS GREATER THAN J AND	5040
C		





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C ***** IN-LINE PROCEDURE FOR ROW AND
C COLUMN EXCHANGE *****
20 SCALE(M) = J
IF (J .EQ. M) GO TO 50
C
DO 30 I = 1, L
F = A(I,J)
A(I,J) = A(I,M)
A(I,M) = F
30 CONTINUE
C
DO 40 I = K, N
F = A(J,I)
A(J,I) = A(M,I)
A(M,I) = F
40 CONTINUE
C
50 GO TO (80,130), IEXC
***** SEARCH FOR ROWS ISOLATING AN EIGENVALUE
AND PUSH THEM DOWN *****
80 IF (L .EQ. 1) GO TO 280
L = L - 1
***** FOR J=L STEP -1 UNTIL 1 DO -- *****
100 DO 120 JJ = 1, L
J = L + 1 - JJ
C
DO 110 I = 1, L
IF (I .EQ. J) GO TO 110
IF (A(J,I) .NE. 0.0) GO TO 120
110 CONTINUE
C
M = L
IEXC = 1
GO TO 20
120 CONTINUE
C

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C      GO TO 140
C      ***** SEARCH FOR COLUMNS ISOLATING AN EIGENVALUE
C      AND PUSH THEM LEFT *****
130 K = K + 1
C
140 DO 170 J = K, L
C
      DO 150 I = K, L
      IF (I.EQ. J) GO TO 150
      IF (A(I,J).NE. 0.0) GO TO 170
150 CONTINUE
C
      M = K
      IEXC = 2
      GO TO 20
170 CONTINUE
C      ***** NOW BALANCE THE SUBMATRIX IN ROWS K TO L *****
      DO 180 I = K, L
      SCALE(I) = 1.0
C      ***** ITERATIVE LOOP FOR NORM REDUCTION *****
190 NOCONV = .FALSE.
C
      DO 270 I = K, L
      C = 0.0
      R = 0.0
C
      DO 200 J = K, L
      IF (J.EQ. I) GO TO 200
      C = C + ABS(A(J,I))
      R = R + ABS(A(I,J))
200 CONTINUE
C
      G = R / RADIX
      F = 1.0
      S = C + R
210 IF (C.GE. G) GO TO 220

```









C	SUBROUTINE ELMHES(NM,N,LOW,IGH,A,INT)	7210
		7220
	INTEGER I,J,M,N,LA,NM,IGH,KPI,LOW,MM1,MP1	7230
	REAL A(NM,N)	7240
	REAL X,Y	7250
	REAL ABS	7260
C	INTEGER INT(IGH)	7270
		7280
C	THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE ELMHES,	7290
C	NUM. MATH. 12, 349-368(1968) BY MARTIN AND WILKINSON.	7300
C	HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 339-358(1971).	7310
C	(REFERENCE 7)	7320
C	GIVEN A REAL GENERAL MATRIX, THIS SUBROUTINE	7330
C	REDUCES A SUBMATRIX SITUATED IN ROWS AND COLUMNS	7340
C	LOW THROUGH IGH TO UPPER HESSENBERG FORM BY	7350
C	STABILIZED ELEMENTARY SIMILARITY TRANSFORMATIONS.	7360
C		7370
C	ON INPUT-	7380
C		7390
C	NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL	7400
C	ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM	7410
C	DIMENSION STATEMENT.	7420
C		7430
C	N IS THE ORDER OF THE MATRIX.	7440
C		7450
C	LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING	7460
C	SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED,	7470
C	SET LOW=1, IGH=N.	7480
C		7490
C	A CONTAINS THE INPUT MATRIX.	7500
C		7510
C	ON OUTPUT-	7520
C		7530
C	A CONTAINS THE HESSENBERG MATRIX. THE MULTIPLIERS	7540
C	WHICH WERE USED IN THE REDUCTION ARE STORED IN THE	7550
C	REMAINING TRIANGLE UNDER THE HESSENBERG MATRIX.	7560

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77720  
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77750  
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77830  
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77850  
77860  
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77900  
77910  
77920

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7930      A(J,I) = A(J,M)
7940      A(J,M) = Y
7950
7960      ***** END INTERCHANGE *****
7970      IF (X .EQ. 0.0) GO TO 180
7980      MP1 = M + I
7990
8000      DO 160 I = MP1, IGH
8010          Y = A(I,MM1)
8020          IF (Y .EQ. 0.0) GO TO 160
8030          Y = Y / X
8040          A(I,MM1) = Y
8050
8060      DO 140 J = M, N
8070          A(I,J) = A(I,J) - Y * A(M,J)
8080
8090      DO 150 J = 1, IGH
8100          A(J,M) = A(J,M) + Y * A(J,I)
8110
8120      CONTINUE
8130
8140      CONTINUE
8150
8160      RETURN
8170      ***** LAST CARD OF ELMHES *****
8180      END
8190
8200      -----
8210
8220      SUBROUTINE HQR(NM,N,LOW,IGH,H,WR,WI,IERR)
8230
8240      INTEGER I,J,K,L,M,N,EN,LL,MM,NA,NM,IGH,ITS,LOW,MP2,ENM2,IERR
8250      REAL H(NM,N),WR(N),WI(N)
8260      REAL P,Q,R,S,T,W,X,Y,ZZ,MACHEP
8270      REAL SQRT,ABS,SIGN
8280      INTEGER MINO

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93





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C          FOR L=EN STEP -1 UNTIL LOW DO -- *****
70 DO 80 LL = LOW, EN
   L = EN + LOW - LL
   IF (L.EQ. LOW) GO TO 100
   IF (ABS(H(L,L-1))) .LE. MACHEP * (ABS(H(L-1,L-1)))
      X = ABS(H(L,L))) GO TO 100
80 CONTINUE
C ***** FORM SHIFT *****
100 X = H(EN,EN)
   IF (L.EQ. EN) GO TO 270
   Y = H(NA,NA)
   W = H(EN,NA) * H(NA,EN)
   IF (L.EQ. NA) GO TO 280
   IF (ITS.EQ. 30) GO TO 1000
   IF (ITS.NE. 10 .AND. ITS.NE. 20) GO TO 130
   ***** FORM EXCEPTIONAL SHIFT *****
   T = T + X
C
C          DO 120 I = LOW, EN
120 H(I,I) = H(I,I) - X
C
C          S = ABS(H(EN,NA)) + ABS(H(NA,ENM2))
   X = 0.75 * S
   Y = X
   W = -0.4375 * S * S
130 ITS = ITS + 1
   ***** LOOK FOR TWO CONSECUTIVE SMALL
   SUB-DIAGONAL ELEMENTS.
   FOR M=EN-2 STEP -1 UNTIL L DO -- *****
C          DO 140 MM = L, ENM2
   M = ENM2 + L - MM
   ZZ = H(M,M)
   R = X - ZZ
   S = Y - ZZ
   P = (R * S - W) / H(M+1,M) + H(M,M+1)
   Q = H(M+1,M+1) - ZZ - R - S

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9370      R = H(M+2,M+1)
9380      S = ABS(P) + ABS(Q) + ABS(R)
9390      P = P / S
9400      Q = Q / S
9410      R = R / S
9420      IF (M .EQ. L) GO TO 150
9430      IF (ABS(H(M,M-1)) * (ABS(Q) + ABS(R)) .LE. MACHEP * ABS(P)
9440      * (ABS(H(M-1,M-1)) + ABS(ZZ) + ABS(H(M+1,M+1)))) GO TO 150
9450      X
9460      140 CONTINUE
9470
9480      C
9490      150 MP2 = M + 2
9500      C
9510      DO 160 I = MP2, EN
9520      H(I,I-2) = 0.0
9530      IF (I .EQ. MP2) GO TO 160
9540      H(I,I-3) = 0.0
9550      160 CONTINUE
9560
9570      C
9580      ***** DOUBLE QR STEP INVOLVING ROWS L TO EN AND
9590      COLUMNS M TO EN *****
9600      DO 260 K = M, NA
9610      NOTLAS = K .NE. NA
9620      IF (K .EQ. M) GO TO 170
9630      P = H(K,K-1)
9640      Q = H(K+1,K-1)
9650      R = 0.0
9660      IF (NOTLAS) R = H(K+2,K-1)
9670      X = ABS(P) + ABS(Q) + ABS(R)
9680      IF (X .EQ. 0.0) GO TO 260
9690      P = P / X
9700      Q = Q / X
9710      R = R / X
9720      S = SIGN(SQRT(P*P+Q*Q+R*R),P)
170      IF (K .EQ. M) GO TO 180
      H(K,K-1) = -S * X
      GO TO 190
180      IF (L .NE. M) H(K,K-1) = -H(K,K-1)

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```

190  P = P + S
    X = P / S
    Y = Q / S
    ZZ = R / S
    Q = Q / P
    R = R / P
    C ***** ROW MODIFICATION *****
      DO 210 J = K, EN
        P = H(K,J) + Q * H(K+1,J)
        IF (.NOT. NOTLAS) GO TO 200
        P = P + R * H(K+2,J)
        H(K+2,J) = H(K+2,J) - P * ZZ
        H(K+1,J) = H(K+1,J) - P * Y
        H(K,J) = H(K,J) - P * X
      200 CONTINUE
      210 CONTINUE
    C
    J = MINO(EN,K+3)
    C ***** COLUMN MODIFICATION *****
      DO 230 I = L, J
        P = X * H(I,K) + Y * H(I,K+1)
        IF (.NOT. NOTLAS) GO TO 220
        P = P + ZZ * H(I,K+2)
        H(I,K+2) = H(I,K+2) - P * R
        H(I,K+1) = H(I,K+1) - P * Q
        H(I,K) = H(I,K) - P
      220 CONTINUE
      230 CONTINUE
    C
    260 CONTINUE
    C
    GO TO 70
    C ***** ONE ROOT FOUND *****
      270 WR(EN) = X + T
        WI(EN) = 0.0
        EN = NA
        GO TO 60
    C ***** TWO ROOTS FOUND *****

```

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9970
9980
9990
10000
10010
10020
10030
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10050
10060
10070
10080

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```

280      P = (Y - X) / 2.0
      Q = P * P + W
      ZZ = SQRT(ABS(Q))
      X = X + T
      IF (Q .LT. 0.0) GO TO 320
      ***** REAL PAIR *****
      ZZ = P + SIGN(ZZ,P)
      WR(NA) = X + ZZ
      WR(EN) = WR(NA)
      IF (ZZ .NE. 0.0) WR(EN) = X - W / ZZ
      WI(NA) = 0.0
      WI(EN) = 0.0
      GO TO 330
      ***** COMPLEX PAIR *****
      WR(NA) = X + P
      WR(EN) = X + P
      WI(NA) = ZZ
      WI(EN) = -ZZ
      EN = ENM2
      GO TO 60
      ***** SET ERROR -- NO CONVERGENCE TO AN
      EIGENVALUE AFTER 30 ITERATIONS *****
      IERR = EN
      RETURN
      ***** LAST CARD OF HQR *****
      END
      -----
      SUBROUTINE HQR2(NM,N,LOW,IGH,H,WR,WI,Z,IERR)
      INTEGER I,J,K,L,M,N,EN,II,JJ,LL,MM,NA,NM,NN,
      X IGH,ITS,LOW,MP2,ENM2,IERR
      REAL H(NM,N),WR(N),WI(N),Z(NM,N)
      REAL P,Q,R,S,T,W,X,Y,RA,SA,VI,VR,ZZ,NORM,MACHEP
      REAL SQRT,ABS,SIGN

```



C	INTEGER MINO	10450
	LOGICAL NOTLAS	10460
C	COMPLEX Z3	10470
	COMPLEX CMPLX	10480
	REAL T3(2)	10490
	EQUIVALENCE (Z3,T3(1))	10500
C		10510
C	THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE HQR2,	10520
C	NUM. MATH. 16, 181-204(1970) BY PETERS AND WILKINSON.	10530
C	HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 372-395(1971).	10540
C	(REFERENCE 7)	10550
C	THIS SUBROUTINE FINDS THE EIGENVALUES AND EIGENVECTORS	10560
C	OF A REAL UPPER HESSENBERG MATRIX BY THE QR METHOD. THE	10570
C	EIGENVECTORS OF A REAL GENERAL MATRIX CAN ALSO BE FOUND	10580
C	IF ELMHES AND ELTRAN OR ORTHES AND ORTRAN HAVE	10590
C	BEEN USED TO REDUCE THIS GENERAL MATRIX TO HESSENBERG FORM	10600
C	AND TO ACCUMULATE THE SIMILARITY TRANSFORMATIONS.	10610
C	ON INPUT -	10620
C		10630
C		10640
C	NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL	10650
C	ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM	10660
C	DIMENSION STATEMENT.	10670
C		10680
C	N IS THE ORDER OF THE MATRIX.	10690
C		10700
C	LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING	10710
C	SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED,	10720
C	SET LOW=1, IGH=N.	10730
C		10740
C	H CONTAINS THE UPPER HESSENBERG MATRIX.	10750
C		10760
C	Z CONTAINS THE TRANSFORMATION MATRIX PRODUCED BY ELTRAN	10770
C	AFTER THE REDUCTION BY ELMHES, OR BY ORTRAN AFTER THE	10780
C	REDUCTION BY ORTHES, IF PERFORMED. IF THE EIGENVECTORS	10790
C	OF THE HESSENBERG MATRIX ARE DESIRED, Z MUST CONTAIN THE	10800

C	IDENTITY MATRIX.	10810
C		10820
C	ON OUTPUT -	10830
C		10840
C	H HAS BEEN DESTROYED.	10850
C		10860
C	WR AND WI CONTAIN THE REAL AND IMAGINARY PARTS,	10870
C	RESPECTIVELY, OF THE EIGENVALUES. THE EIGENVALUES	10880
C	ARE UNORDERED EXCEPT THAT COMPLEX CONJUGATE PAIRS	10890
C	OF VALUES APPEAR CONSECUTIVELY WITH THE EIGENVALUE	10900
C	HAVING THE POSITIVE IMAGINARY PART FIRST. IF AN	10910
C	ERROR EXIT IS MADE, THE EIGENVALUES SHOULD BE CORRECT	10920
C	FOR INDICES IERR+1,...,N.	10930
C		10940
C	Z CONTAINS THE REAL AND IMAGINARY PARTS OF THE EIGENVECTORS,	10950
C	IF THE I-TH EIGENVALUE IS REAL, THE I-TH COLUMN OF Z	10960
C	CONTAINS ITS EIGENVECTOR. IF THE I-TH EIGENVALUE IS COMPLEX	10970
C	WITH POSITIVE IMAGINARY PART, THE I-TH AND (I+1)-TH	10980
C	COLUMNS OF Z CONTAIN THE REAL AND IMAGINARY PARTS OF ITS	10990
C	EIGENVECTOR. THE EIGENVECTORS ARE UNNORMALIZED. IF AN	11000
C	ERROR EXIT IS MADE, NONE OF THE EIGENVECTORS HAS BEEN FOUND.	11010
C		11020
C	IERR IS SET TO	11030
C	ZERO FOR NORMAL RETURN,	11040
C	J IF THE J-TH EIGENVALUE HAS NOT BEEN	11050
C	DETERMINED AFTER 30 ITERATIONS.	11060
C		11070
C	ARITHMETIC IS REAL EXCEPT FOR THE REPLACEMENT OF THE ALGOL	11080
C	PROCEDURE CDIV BY COMPLEX DIVISION.	11090
C		11100
C		11110
C		11120
C		11130
C		11140
C		11150
C		11160

	-----
*****	MACHEP IS A MACHINE DEPENDENT PARAMETER SPECIFYING
C	

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C                                     THE RELATIVE PRECISION OF FLOATING POINT ARITHMETIC.
C
C                                     *****
C                                     MACHEP = 2.**(-47)
C
C                                     IERR = 0
C                                     ***** STORE ROOTS ISOLATED BY BALANC *****
C                                     DO 50 I = 1, N
C                                     IF (I .GE. LOW .AND. I .LE. IGH) GO TO 50
C                                     WR(I) = H(I,I)
C                                     WI(I) = 0.0
C                                     50 CONTINUE
C
C                                     EN = IGH
C                                     T = 0.0
C                                     ***** SEARCH FOR NEXT EIGENVALUES *****
C                                     60 IF (EN .LT. LOW) GO TO 340
C                                     ITS = 0
C                                     NA = EN - 1
C                                     ENM2 = NA - 1
C                                     ***** LOOK FOR SINGLE SMALL SUB-DIAGONAL ELEMENT
C                                     FOR L=EN STEP -1 UNTIL LOW DO -- *****
C                                     70 DO 80 LL = LOW, EN
C                                     L = EN + LOW - LL
C                                     IF (L .EQ. LOW) GO TO 100
C                                     IF (ABS(H(L,L-1)) .LE. MACHEP * (ABS(H(L-1,L-1))
C                                     X + ABS(H(L,L)))) GO TO 100
C                                     80 CONTINUE
C                                     ***** FORM SHIFT *****
C                                     100 X = H(EN,EN)
C                                     IF (L .EQ. EN) GO TO 270
C                                     Y = H(NA,NA)
C                                     W = H(EN,NA) * H(NA,EN)
C                                     IF (L .EQ. NA) GO TO 280
C                                     IF (ITS .EQ. 30) GO TO 1000
C                                     IF (ITS .NE. 10 .AND. ITS .NE. 20) GO TO 130

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C          ***** FORM EXCEPTIONAL SHIFT *****
C          T = T + X
C          DO 120 I = LOW, EN
C            120 H(I,I) = H(I,I) - X
C          S = ABS(H(EN,NA)) + ABS(H(NA,ENM2))
C          X = 0.75 * S
C          Y = X
C          W = -0.4375 * S * S
C          130 ITS = ITS + 1
C          ***** LOOK FOR TWO CONSECUTIVE SMALL
C          SUB-DIAGONAL ELEMENTS.
C          FOR M=EN-2 STEP -1 UNTIL L DO -- *****
C            DO 140 MM = L, ENM2
C              M = ENM2 + L - MM
C              ZZ = H(M,M)
C              R = X - ZZ
C              S = Y - ZZ
C              P = (R * S - W) / H(M+1,M) + H(M,M+1)
C              Q = H(M+1,M+1) - ZZ - R - S
C              R = H(M+2,M+1)
C              S = ABS(P) + ABS(Q) + ABS(R)
C              P = P / S
C              Q = Q / S
C              R = R / S
C              IF (M.EQ. L) GO TO 150
C              IF (ABS(H(M,M-1)) * (ABS(Q) + ABS(R)) .LE. MACHEP * ABS(P))
C                X * (ABS(H(M-1,M-1)) + ABS(ZZ) + ABS(H(M+1,M+1))) GO TO 150
C            140 CONTINUE
C          150 MP2 = M + 2
C          DO 160 I = MP2, EN
C            H(I,I-2) = 0.0
C            IF (I.EQ. MP2) GO TO 160

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11890
11900
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11980
11990
12000
12010
12020
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12050
12060
12070
12080
12090
12100
12110
12120
12130
12140
12150
12160
12170
12180
12190
12200
12210
12220
12230
12240

160      H(I,I-3) = 0.0
      CONTINUE
C ***** DOUBLE QR STEP INVOLVING ROWS L TO EN AND
C          COLUMNS M TO EN *****
      DO 260 K = M, NA
        NOTLAS = K .NE. NA
        IF (K .EQ. M) GO TO 170
        P = H(K,K-1)
        Q = H(K+1,K-1)
        R = 0.0
        IF (NOTLAS) R = H(K+2,K-1)
        X = ABS(P) + ABS(Q) + ABS(R)
        IF (X .EQ. 0.0) GO TO 260
        P = P / X
        Q = Q / X
        R = R / X
        S = SIGN(SQRT(P*P+Q*Q+R*R),P)
        IF (K .EQ. M) GO TO 180
        H(K,K-1) = -S * X
        GO TO 190
170      IF (L .NE. M) H(K,K-1) = -H(K,K-1)
180      P = P + S
190      X = P / S
        Y = Q / S
        ZZ = R / S
        Q = Q / P
        R = R / P
C ***** ROW MODIFICATION *****
      DO 210 J = K, N
        P = H(K,J) + Q * H(K+1,J)
        IF (.NOT. NOTLAS) GO TO 200
        P = P + R * H(K+2,J)
        H(K+2,J) = H(K+2,J) - P * ZZ
        H(K+1,J) = H(K+1,J) - P * Y
        H(K,J) = H(K,J) - P * X
200      CONTINUE
210

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C
H(NA,NA) = Y + T
IF (Q .LT. 0.0) GO TO 320
***** REAL PAIR *****
ZZ = P + SIGN(ZZ,P)
WR(NA) = X + ZZ
WR(EN) = WR(NA)
IF (ZZ .NE. 0.0) WR(EN) = X - W / ZZ
WI(NA) = 0.0
WI(EN) = 0.0
X = H(EN,NA)
R = SQRT(X*X+ZZ*ZZ)
P = X / R
Q = ZZ / R
***** ROW MODIFICATION *****
DO 290 J = NA, N
  ZZ = H(NA,J)
  H(NA,J) = Q * ZZ + P * H(EN,J)
  H(EN,J) = Q * H(EN,J) - P * ZZ
290 CONTINUE
***** COLUMN MODIFICATION *****
DO 300 I = 1, EN
  ZZ = H(I,NA)
  H(I,NA) = Q * ZZ + P * H(I,EN)
  H(I,EN) = Q * H(I,EN) - P * ZZ
300 CONTINUE
***** ACCUMULATE TRANSFORMATIONS *****
DO 310 I = LOW, IGH
  ZZ = Z(I,NA)
  Z(I,NA) = Q * ZZ + P * Z(I,EN)
  Z(I,EN) = Q * Z(I,EN) - P * ZZ
310 CONTINUE
C
GO TO 330
C
***** COMPLEX PAIR *****
320 WR(NA) = X + P
  WR(EN) = X + P
12610
12620
12630
12640
12650
12660
12670
12680
12690
12700
12710
12720
12730
12740
12750
12760
12770
12780
12790
12800
12810
12820
12830
12840
12850
12860
12870
12880
12890
12900
12910
12920
12930
12940
12950
12960

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12970      WI(NA) = ZZ
12980      WI(EN) = -ZZ
12990      330 EN = ENM2
13000      GO TO 60
13010      C ***** ALL ROOTS FOUND, BACKSUBSTITUTE TO FIND
13020      C VECTORS OF UPPER TRIANGULAR FORM *****
13030      340 NORM = 0.0
13040      K = 1
13050      C
13060      DO 360 I = 1, N
13070      C
13080      DO 350 J = K, N
13090      350 NORM = NORM + ABS(H(I,J))
13100      C
13110      K = I
13120      360 CONTINUE
13130      C
13140      IF (NORM .EQ. 0.0) GO TO 1001
13150      C ***** FOR EN=N STEP -1 UNTIL 1 DO -- *****
13160      DO 800 NN = 1, N
13170      EN = N + 1 - NN
13180      P = WR(EN)
13190      Q = WI(EN)
13200      NA = EN - 1
13210      IF (Q) 710, 600, 800
13220      C ***** REAL VECTOR *****
13230      600 M = EN
13240      H(EN, EN) = 1.0
13250      IF (NA .EQ. 0) GO TO 800
13260      C ***** FOR I=EN-1 STEP -1 UNTIL 1 DO -- *****
13270      DO 700 II = 1, NA
13280      I = EN - II
13290      W = H(I,I) - P
13300      R = H(I,EN)
13310      IF (M .GT. NA) GO TO 620
13320      C

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```

        DO 610 J = M, NA
        R = R + H(I,J) * H(J,EN)
C
        610
        IF (WI(I) .GE. 0.0) GO TO 630
        ZZ = W
        S = R
        GO TO 700
        620
        M = I
        IF (WI(I) .NE. 0.0) GO TO 640
        T = W
        IF (W .EQ. 0.0) T = MACHEP * NORM
        H(I,EN) = -R / T
        GO TO 700
        630
C ***** SOLVE REAL EQUATIONS *****
        640
        X = H(I,I+1)
        Y = H(I+1,I)
        Q = (WR(I) - P) * (WR(I) - P) + WI(I) * WI(I)
        T = (X * S - ZZ * R) / Q
        H(I,EN) = T
        IF (ABS(X) .LE. ABS(ZZ)) GO TO 650
        H(I+1,EN) = (-R - W * T) / X
        GO TO 700
        650
        H(I+1,EN) = (-S - Y * T) / ZZ
        700
        CONTINUE
C ***** END REAL VECTOR *****
        GO TO 800
C ***** COMPLEX VECTOR *****
        710
        M = NA
C *****
C ***** LAST VECTOR COMPONENT CHOSEN IMAGINARY SO THAT
C ***** EIGENVECTOR MATRIX IS TRIANGULAR *****
        IF (ABS(H(EN,NA)) .LE. ABS(H(NA,EN))) GO TO 720
        H(NA,NA) = Q / H(EN,NA)
        H(NA,EN) = -(H(EN,EN) - P) / H(EN,NA)
        GO TO 730
        720
        Z3 = CMPLX(0.0, -H(NA,EN)) / CMPLX(H(NA,NA) - P, Q)
        H(NA,NA) = T3(1)

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```

730      H(NA,EN) = T3(2)
          H(EN,NA) = 0.0
          H(EN,EN) = 1.0
          ENM2 = NA - 1
          IF (ENM2 .EQ. 0) GO TO 800

C
      DO 790 II = 1, ENM2
        I = NA - II
        W = H(I,I) - P
        RA = 0.0
        SA = H(I,EN)

C
      DO 760 J = M, NA
        RA = RA + H(I,J) * H(J,NA)
        SA = SA + H(I,J) * H(J,EN)
        CONTINUE
760
C
      IF (WI(I) .GE. 0.0) GO TO 770
      ZZ = W
      R = RA
      S = SA
      GO TO 790
      M = I
770
      IF (WI(I) .NE. 0.0) GO TO 780
      Z3 = CMPLX(-RA,-SA) / CMPLX(W,Q)
      H(I,NA) = T3(1)
      H(I,EN) = T3(2)
      GO TO 790

C ***** SOLVE COMPLEX EQUATIONS *****
780      X = H(I,I+1)
          Y = H(I+1,I)
          VR = (WR(I) - P) * (WR(I) - P) + WI(I) * WI(I) - Q * Q
          VI = (WR(I) - P) * 2.0 * Q
          IF (VR .EQ. 0.0 .AND. VI .EQ. 0.0) VR = MACHEP * NORM1
              * (ABS(W) + ABS(Q) + ABS(X) + ABS(Y) + ABS(ZZ))
          Z3 = CMPLX(X*R-ZZ*RA+Q*SA,X*S-ZZ*SA-Q*RA) / CMPLX(VR,VI)
          X

```



```

14050 H(I,NA) = T3(1)
14060 H(I,EN) = T3(2)
14070 IF (ABS(X) .LE. ABS(ZZ) + ABS(Q)) GO TO 785
14080 H(I+1,NA) = (-RA - W * H(I,NA) + Q * H(I,EN)) / X
14090 H(I+1,EN) = (-SA - W * H(I,EN) - Q * H(I,NA)) / X
14100 GO TO 790
14110 Z3 = CMPLX(-R-Y*H(I,NA), -S-Y*H(I,EN)) / CMPLX(ZZ,Q)
14120 H(I+1,NA) = T3(1)
14130 H(I+1,EN) = T3(2)
14140
14150
14160
14170
14180
14190
14200
14210
14220
14230
14240
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14400

785
C ***** END COMPLEX VECTOR *****
790 CONTINUE
C ***** END BACK SUBSTITUTION.
C ***** VECTORS OF ISOLATED ROOTS *****
C DO 840 I = 1, N
C IF (I .GE. LOW .AND. I .LE. IGH) GO TO 840
C
C DO 820 J = I, N
C Z(I,J) = H(I,J)
C
C 840 CONTINUE
C ***** MULTIPLY BY TRANSFORMATION MATRIX TO GIVE
C VECTORS OF ORIGINAL FULL MATRIX.
C FOR J=N STEP -1 UNTIL LOW DO -- *****
C DO 880 JJ = LOW, N
C J = N + LOW - JJ
C M = MINO(J,IGH)
C
C DO 880 I = LOW, IGH
C ZZ = 0.0
C
C DO 860 K = LOW, M
C ZZ = ZZ + Z(I,K) * H(K,J)
C
C Z(I,J) = ZZ
880 CONTINUE

```





```

100      Z(I,MP) = A(I,MP-1)
C
      I = INT(MP)
      IF (I .EQ. MP) GO TO 140
C
      DO 130 J = MP, IGH
        Z(MP,J) = Z(I,J)
        Z(I,J) = 0.0
      130 CONTINUE
C
      Z(I,MP) = 1.0
      140 CONTINUE
C
      200 RETURN
C ***** LAST CARD OF ELTRAN *****
      END
      SUBROUTINE RGEIG (M,N,A,IND,EIGVAL,EIGVEC,WORK)

      EIGENVALUES AND (OPTIONALLY) EIGENVECTORS OF
      REAL GENERAL MATRIX.

      CALL RGEIG (M,N,A,IND,EIGVAL,EIGVEC,WORK)

      INPUT PARAMETERS
      M      ROW DIMENSION OF A
      N      ORDER OF MATRIX A. N MUST BE LESS THAN OR EQUAL
              TO M.
      A      INPUT MATRIX, REAL, TWO-DIMENSIONAL
              A(M,K) WHERE K IS GREATER THAN OR EQUAL TO N,
      IND     ZERO IF ONLY EIGENVALUES ARE TO BE COMPUTED
              NON ZERO OTHERWISE.
      WORK   A WORKING STORAGE AREA OF DIMENSION 2*N

      OUTPUT PARAMETERS

```

C	A	IS DESTROYED.	15490
C	IND	IF MORE THAN 30 ITERATIONS ARE REQUIRED TO DETERMINE	15500
C		AN EIGENVALUE, THIS SUBROUTINE TERMINATES WITH IND	15510
C		SET TO THE INDEX OF THE EIGENVALUE FOR WHICH THE	15520
C		FAILURE OCCURS. THE EIGENVALUES IN THE EIGVAL VECTOR	15530
C		SHOULD BE CORRECT FOR INDICES IND+1, IND+2, .... N	15540
C		BUT NO EIGENVECTORS ARE COMPUTED. IF ALL THE EIGEN-	15550
C		VALUES ARE DETERMINED WITHIN 30 ITERATIONS, IND IS	15560
C		SET TO ZERO. IF N IS GREATER THAN M, IND IS SET TO	15570
C		N+1 AS AN ERROR FLAG.	15580
C	EIGVAL	COMPLEX N-VECTOR OF EIGENVALUES.	15590
C	EIGVEC	COMPLEX TWO-DIMENSIONAL VARIABLE WITH ROW DIMENSION	15600
C		M AND COLUMN DIMENSION AT LEAST N. EIGVEC CONTAINS	15610
C		THE N EIGENVECTORS. IF IND IS ZERO, EIGVEC IS NOT	15620
C		USED.	15630
C			15640
C			15650
C			15660
C			15670
C	REMARKS	THE EIGENVALUES ARE UNORDERED EXCEPT THAT COMPLEX	15680
C		CONJUGATE PAIRS OF EIGENVALUES APPEAR CONSECUTIVELY	15690
C		WITH THE EIGENVALUE HAVING THE POSITIVE IMAGINARY	15700
C		PART FIRST.	15710
C		EIGENVECTORS ARE STORED COLUMNWISE IN EIGVEC SO THAT THE	15720
C		EIGENVECTOR CORRESPONDING TO THE J-TH EIGENVALUE IS	15730
C		STORED IN THE J-TH COLUMN OF THE COMPLEX MATRIX EIGVEC.	15740
C		EACH EIGENVECTOR IS NORMALIZED SO THAT THE ELEMENT	15750
C		WITH THE LARGEST MODULUS IS REDUCED TO ONE.	15760
C	METHOD		15770
C		THE INPUT MATRIX IS REDUCED TO UPPER HESSENBERG FORM	15780
C		USING ELEMENTARY TRANSFORMATIONS, THE EIGENVALUES OF	15790
C		THE REDUCED MATRIX ARE OBTAINED BY THE QR-ALGORITHM.	15800
C		THE EIGENVECTORS ARE DETERMINED BY BACK-SUBSTITUTION AND	15810
C		THEN TRANSFORMED TO THE EIGENVECTORS OF THE ORIGINAL	15820
C		MATRIX USING THE SIMILARITY TRANSFORMATIONS OF THE	15830
C		ITERATIONS.	15840



C	15850	
C	15860	
C	15870	
C	15880	
C	15890	
C	15900	
C	15910	
C	15920	
C	15930	
C	15940	
C	15950	
	15960	
	15970	
	15980	
	15990	
	16000	
	16010	
	16020	
	16030	
	16040	
	16050	
	16060	
	16070	
	16080	
	16090	
	16100	
	16110	
	16120	
	16130	
	16140	
	16150	
	16160	
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	16180	
	16190	
	16200	

```

REFERENCES
SEE DOCUMENTATION OF EISPACK ROUTINES BALANC, ELMHES,
ELTRAN, HQR2, HQR, AND BALBAK,

THIS PROGRAM USES SUBROUTINES BALANC, ELMHES, ELTRAN,
HQR2, HQR, AND BALBAK WHICH WERE OBTAINED FROM ARGONNE
NATIONAL LABORATORY AS PART OF THE EISPACK SUBROUTINE
PACKAGE. (REFERENCE 8)

COMPLEX B,DENOM
DIMENSION A(M,N), EIGVAL(2,N), EIGVEC(2,M,N), WORK (1)

IF (N.GT.M) GO TO 20
INT = N
INTP1 = INT + 1
CALL BALANC(M,N,A,LOW,IGH,WORK)
CALL ELMHES(M,N,LOW,IGH,A,WORK(INTP1))
IF (IND.EQ.0) GO TO 9
CALL ELTRAN(M,N,LOW,IGH,A,WORK(INTP1),EIGVEC)
IND = 0
CALL HQR2(M,N,LOW,IGH,A,EIGVAL,EIGVAL(N+1),EIGVEC,IND)
L = N
IF (IND.NE.0) RETURN
CALL BALBAK(M,N,LOW,IGH,WORK,L,EIGVEC)
DO 1 I=1,N
  WORK(I) = EIGVAL(N+1)
1 CONTINUE
DO 2 I=1,N
  EIGVAL(2,N-I+1) = WORK(N-I+1)
  EIGVAL(1,N-I+1) = EIGVAL(N-I+1)
2 CONTINUE
MN = M*N
DO 3 I=1,MN
  A(I) = EIGVEC(I)
3 CONTINUE

```

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16210
16220
16230
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16390
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16450
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16500
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16550
16560

!

      K = 1
      4 IF (EIGVAL(2,K).NE.0.0) GO TO 6
        DO 5 J=1,M
          EIGVEC(2,J,K) = 0.0
          EIGVEC(1,J,K) = A(J,K)
        5 CONTINUE
        GO TO 8
      6 DO 7 J=1,M
        EIGVEC(2,J,K) = A(J,K+1)
        EIGVEC(2,J,K+1) = -A(J,K+1)
        EIGVEC(1,J,K) = A(J,K)
        EIGVEC(1,J,K+1) = A(J,K)
      7 CONTINUE
      K = K+1
      8 K = K+1
      IF (K.LE.N) GO TO 4
      GO TO 12
    9 CALL HQR(M,N,LOW,IGH,A,EIGVAL,EIGVAL(N+1),IND)
      DO 10 I=1,N
        WORK(I) = EIGVAL(N+I)
      10 CONTINUE
      DO 11 I=1,N
        EIGVAL(2,N-I+1) = WORK(N-I+1)
        EIGVAL(1,N-I+1) = EIGVAL(N-I+1)
      11 CONTINUE
      RETURN
    12 DO 15 J=1,N
      CMOD = 0.0
      DO 13 I=1,N
        B = CMPLX(EIGVEC(1,I,J),EIGVEC(2,I,J))
        TEMP = CABS(B)
        IF(TEMP.LE.CMOD) GO TO 13
        K = I
        CMOD = TEMP
      13 CONTINUE
      DENOM = CMPLX(EIGVEC(1,K,J),EIGVEC(2,K,J))

```

16570  
16580  
16590  
16600  
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16620  
16630  
16640  
16550  
16660  
16670

DO 14 I=1,N  
B = CMPLX(EIGVEC(1,I,J),EIGVEC(2,I,J))  
B = B/DENOM  
EIGVEC(1,I,J) = REAL(B)  
EIGVEC(2,I,J) = AIMAG(B)  
14 CONTINUE  
15 CONTINUE  
RETURN  
20 IND = N + 1  
RETURN  
END

APPENDIX C  
INTERMEDIATE MATRICES (FOR CASE A)

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1.22767019E+06	M=SUB-L	1.26767161E+06	-1.45002533E+05
1.7699562E+05		-6.7813792E-09	1.58123755E+04
1.5482557E+06		5.77533181E-05	1.12822423E+08
2.72271204E+06		8.11147757E-05	5.01448180E+06
3.1045502E+05		-6.65833787E-05	0.10586994E+04
4.16471809E+01		-1.73303606E-01	1.00000000E+00
5.2929292E+01		1.00000000E+00	-9.2838458E+01
4.2000000E+02		-6.6060051E-01	-6.42065207E+01
6.2317860E+02		-9.27900247E-01	-2.94970812E+02
1.0000000E+00		7.6170350E-01	6.19370934E+01
3.6243303E+03		1.54267157E-04	2.14156621E+03
1.2404540E+03		-2.07622581E-05	-1.69070928E+04
4.58701952E+04		2.79031714E-06	1.02805701E+05
1.6814255E+04	F=SUB-L	-3.76077027E-07	-7.08992724E+07
1.58868070E+01	T	5.1021049E+01	7.04506734E+01
1.70000000E+06	M	8.50000000E+04	8.50000000E+04
1.74750000E+06		-8.92500000E+04	-2.97500000E+04
8.26493809E+02	ALPHA	1.13427469E+03	1.52456002E+03
2.741080770E+01		1.23395522E+00	8.83955224E+01
		U=SUB-L(1)	
		U=SUB-L(2)	
		U=SUB-L(3)	
		U=SUB-L(4)	
		U=SUB-L(5)	
		U=SUB-L(6)	
		U=SUB-L(7)	
		U=SUB-L(8)	
		U=SUB-L(9)	
		U=SUB-L(10)	
		U=SUB-L(11)	
		U=SUB-L(12)	
		U=SUB-L(13)	
		U=SUB-L(14)	
		U=SUB-L(15)	
		U=SUB-L(16)	
		U=SUB-L(17)	
		U=SUB-L(18)	
		U=SUB-L(19)	
		U=SUB-L(20)	
		U=SUB-L(21)	
		U=SUB-L(22)	
		U=SUB-L(23)	
		U=SUB-L(24)	
		U=SUB-L(25)	
		U=SUB-L(26)	
		U=SUB-L(27)	
		U=SUB-L(28)	
		U=SUB-L(29)	
		U=SUB-L(30)	
		U=SUB-L(31)	
		U=SUB-L(32)	
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		U=SUB-L(40)	
		U=SUB-L(41)	
		U=SUB-L(42)	
		U=SUB-L(43)	
		U=SUB-L(44)	
		U=SUB-L(45)	
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		U=SUB-L(47)	
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		U=SUB-L(67)	
		U=SUB-L(68)	
		U=SUB-L(69)	
		U=SUB-L(70)	
		U=SUB-L(71)	
		U=SUB-L(72)	
		U=SUB-L(73)	
		U=SUB-L(74)	
		U=SUB-L(75)	
		U=SUB-L(76)	
		U=SUB-L(77)	
		U=SUB-L(78)	
		U=SUB-L(79)	
		U=SUB-L(80)	
		U=SUB-L(81)	
		U=SUB-L(82)	
		U=SUB-L(83)	
		U=SUB-L(84)	
		U=SUB-L(85)	
		U=SUB-L(86)	
		U=SUB-L(87)	
		U=SUB-L(88)	
		U=SUB-L(89)	
		U=SUB-L(90)	
		U=SUB-L(91)	
		U=SUB-L(92)	
		U=SUB-L(93)	
		U=SUB-L(94)	
		U=SUB-L(95)	
		U=SUB-L(96)	
		U=SUB-L(97)	
		U=SUB-L(98)	
		U=SUB-L(99)	
		U=SUB-L(100)	

BEST AVAILABLE COPY

BEST AVAILABLE COPY

B MATRIX											
1	1.27967019E-06	-6.8235836E-07	1.25767161E-06	-1.45002533E-05	2.07951621E-07	-2.05038163E-07	6.5603570E-07				
2	-1.82719071E-05	1.00000000E+00	0.	0.	-2.41044377E-01	0.	0.				
3	1.1699140E+05	3.11699140E+05	-8.78137929E-05	1.59323755E-04	2.04453777E-06	1.32753714E-05	-6.5994188E-05				
4	1.1210115E+04	1.00000000E+00	0.	0.	1.8955222E-01	0.	8.2500408E+02				
5	1.89160000E+05	6.89160000E+05	5.77333181E-05	1.12042523E-04	-2.68253974E-07	-0.93519411E-05	2.94561450E+05				
6	1.00000000E+05	1.00000000E+00	0.	0.	5.33055222E-01	0.	1.1327260E+03				
7	2.2271204E-06	4.45436234E-05	8.11147757E-05	5.01408380E-06	4.59461907E-07	2.76171149E-05	4.1355101E+05				
8	1.00000000E+00	1.00000000E+00	0.	0.	8.8955222E-01	0.	1.302847E+03				
9	1.421820E-05	-2.1421820E-05	-6.5833787E-05	-1.05689907E-04	-5.8401653E-04	-9.12362750E-06	-1.3971538E+05				
10	1.00000000E+05	1.00000000E+00	1.54267157E-04	2.16156621E-03	1.23305271E-04	-0.6783812E-05	-1.5256742E+03				
11	2.27243997E-04	0.	0.	1.00000000E+00	6.1244181E-04	0.	0.				
12	1.28083800E+03	0.	-2.07622581E-05	-1.49070928E-04	0.	0.	0.				
13	1.05003006E-04	0.	0.	0.	2.17455060E-04	-1.65214545E-05	-1.05931220E+05				
14	1.5114255E-04	-1.13046820E-06	-3.76077027E-07	-7.08992723E-07	1.00000000E+00	0.	0.				
15	-1.2212683E-07	0.	0.	0.	2.75622471E-05	-0.01470190E-07	-1.91478447E+07				
16	2.73151621E-07	-2.05038163E-07	4.59983576E-07	-1.02712071E-05	0.	0.	1.00000000E+00				
17	1.07000000E+00	1.07000000E+00	5.00000000E+00	0.	-2.41044377E-01	-4.8235836E-07	1.25767161E-06				
18	1.32753714E-05	1.32753714E-05	-8.78137929E-05	1.12148115E-04	1.76876543E-05	-1.20522388E+00	-3.01360971E+00				
19	1.89160000E+05	5.00000000E+00	2.94561450E-05	0.	1.8955222E-01	3.11699140E-05	-8.78137929E-05				
20	2.93519411E-05	1.00000000E+00	5.00000000E+00	8.00024680E-05	-1.8955222E-01	9.10776119E-01	8.27709220E+02				
21	1.00000000E+05	1.00000000E+00	5.00000000E+00	0.	5.33055222E-01	-0.87166744E-04	5.77333181E-05				
22	2.76171149E-05	4.1355101E-05	4.1355101E-05	3.55227650E-06	2.72271204E-06	2.68277612E+00	1.10000000E+03				
23	1.00000000E+00	1.00000000E+00	5.00000000E+00	0.	8.8955222E-01	6.44036234E-05	8.11147757E-05				
24	-9.12362750E-06	-3.3971538E-05	-7.48451291E-05	-7.48451291E-05	4.59461907E-07	4.4977612E+00	1.39193014E+03				
25	1.00000000E+00	1.00000000E+00	5.00000000E+00	0.	1.23305271E-04	-2.1421820E-05	-6.5533787E-05				
26	1.00000000E+00	1.00000000E+00	5.00000000E+00	-1.05593908E-04	1.23305271E-04	6.18977612E+00	1.53994565E+03				
27	-1.65214545E-05	-1.05593908E-04	0.	0.	1.23305271E-04	-3.14792404E-05	-2.07622581E-05				
28	-1.65214545E-04	0.	0.	0.	1.00000000E+00	5.00000000E+00	1.25000000E+01				
C											
1	-1.92000000E-04	0.	0.	0.	0.	0.	0.				
2	5.26300271E-01	2.0275370E-01	2.06800001E+00	-9.79123123E-01	5.26300271E-01	2.0275370E-01	2.06800001E+00				
3	2.04000000E-04	-8.78990970E-18	-4.13303147E-03	-2.03851051E-03	4.00696042E-06	-1.7627457E-06	0.				

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0.37041932E-01	P-SUR=L ( 1 )	9.66105978E-01	-1.20522388E-01	-3.01305970E-02
2.01773047E-01	P-SUR=G ( 1 )	7.33198062E-01	9.19776119E-02	8.25517803E+02
1.70000000E+00	V-SUR=AU ( 1 )	-2.41044774E-01	2.66977612E-01	1.13634143E+03
1.70000000E+00	0.	1.63955224E-01	4.41977612E-01	1.34009119E+03
1.70000000E+00	0.	5.33955224E-01	6.16977612E-01	1.52471468E+03
1.70000000E+00	0.	8.63955224E-01	-2.41044776E+05	-1.20522388E+05
0.	V-SUR=AM ( 1 )	1.23395922E+00	1.56361940E+04	7.41409701E+03
0.	0.	0.	4.53461940E+04	2.26930970E+04
0.	0.	0.	7.51361940E+04	3.75467070E+04
0.	0.	0.	1.04486194E+05	5.24430970E+04
0.	V-SUR=AV ( 1 )	5.00000000E+01	1.25000000E-01	2.04333333E-02
0.	0.	1.00000000E+00	5.00000000E-01	1.25000000E-01
0.	V-SUR=ALPHA ( 1 )	0.	1.00000000E+00	5.00000000E-01
0.	0.	0.	0.	1.00000000E+00
0.	V-SUR=ALPHA ( 1 )	0.	0.	1.00000000E+00
0.	0.	0.	0.	0.

7.0000000E+01	F-SUB=L ( 2 )	9.3330000E-01	-2.41004776E+01	-1.20522300E-01
2.0100000E+01	F-SUB=G ( 2 )	7.50021250E-01	1.83955220E+01	8.2550770E+02
1.0000000E+00	V-SUB=AU ( 2 )	-2.41004776E-01	5.33955220E-01	1.13054107E+03
1.0000000E+00		1.83955220E+01	8.83955220E-01	1.30132200E+03
1.0000000E+00		5.33955220E-01	1.23395522E+00	1.52517730E+03
1.0000000E+00		8.83955220E-01	-2.41004776E+03	-2.41004776E+03
1.0000000E+00	V-SUB=AM ( 2 )	1.23395522E+00	1.56361900E+04	1.56361900E+04
0.0		0.	4.53061900E+04	4.53061900E+04
0.0		0.	7.51361900E+04	7.51361900E+04
0.0		0.	1.00000000E+05	1.00000000E+05
0.0	V-SUB=AV ( 2 )	1.00000000E+00	5.00000000E-01	1.60000000E+01
0.0	V-SUB=ALPHA ( 2 )	1.00000000E+00	1.00000000E+00	5.00000000E-01
0.0	V-SUB=BETA ( 2 )	0.	1.00000000E+00	1.00000000E+00
0.0	V-SUB=PHI ( 2 )	0.	0.	1.00000000E+00



50639660E-01	F-SUR-L ( 3 )	7.74093310E-01	0.17193337E-01	9.01724892E-01	-3.61597168E-01	-2.71173373E-01
20794387E-01	F-SUR-G ( 3 )	5.50196399E-01	6.24344907E-01	7.85546917E-01	2.75932036E-01	0.25702754E+02
100000000E+00	V-SUR-AU ( 3 )	1.50000000E+00	0.	-2.41044776E-01	0.00932436E-01	1.11447519E+03
100000000E+00		1.50000000E+00	0.	1.83955224E-01	1.32591244E+00	1.38167515E+03
100000000E+00		1.50000000E+00	0.	5.33955224E-01	1.09093284E+00	1.52594462E+03
100000000E+00		1.50000000E+00	0.	8.83955224E-01	1.23394522E+00	1.52594462E+03
100000000E+00	V-SUR-AM ( 3 )	1.50000000E+00	0.	1.23394522E+00	-2.41044776E+05	-3.61597168E+05
07		1.00000000E+06	0.	0.	1.56361940E+04	2.34542910E+04
07		0.50000000E+04	0.	0.	8.53861940E+04	6.80792910E+04
07		0.50000000E+04	0.	0.	7.51361940E+04	1.12702291E+05
07		0.50000000E+04	0.	0.	1.04486194E+05	1.57329291E+05
07	V-SUR-AV ( 3 )	0.50000000E+04	0.	0.	1.12500000E+00	5.62450000E-01
07	V-SUR-A TME TA ( 3 )	0.	1.00000000E+00	1.50000000E+00	1.50000000E+00	1.12400000E+00
07		0.	0.	1.00000000E+00	1.00000000E+00	1.50000000E+00
07	V-SUR-A BETA ( 3 )	0.	0.	0.	1.00000000E+00	1.50000000E+00
07	V-SUR-A PHI ( 3 )	0.	0.	0.	0.	1.00000000E+00

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0.1002249E+01	F=SUB=L ( 5 )	0.4163337E-01	-6.0261190E-01	-7.5326492E-01
0.1002249E+01	F=SUB=G ( 5 )	0.4163337E-01	0.5000000E+01	0.2607069E+02
0.1002249E+01	F=SUB=H ( 5 )	0.4163337E-01	1.3348840E+00	1.1350433E+03
1.0000000E+00	Y=SUB=AU ( 5 )	-2.4104477E-01	2.2098840E+00	1.3836430E+03
1.0000000E+00	0.	1.8395522E-01	3.0848840E+00	1.5284165E+03
1.0000000E+00	0.	9.3395522E-01	-2.4104477E+05	-4.0261190E+05
1.0000000E+00	0.	0.4395522E-01	1.5636194E+04	3.0090465E+04
1.0000000E+00	Y=SUB=AM ( 5 )	1.23395522E+00	4.5386194E+04	1.1345485E+05
0.	0.	0.	7.5136194E+04	1.8744045E+05
0.	0.	0.	1.0488019E+05	2.6221545E+05
0.	Y=SUB=AV ( 5 )	0.	3.1350000E+00	2.6041645E+00
0.	Y=SUB=A YHETA ( 5 )	2.5000000E+00	2.5000000E+00	3.1250000E+00
0.	Y=SUB=A BETA ( 5 )	1.0000000E+00	1.0000000E+00	2.5000000E+00
0.	Y=SUB=A PHI ( 5 )	0.	0.	1.0000000E+00
0.	0.	0.	0.	0.

37.44013440E-01	P-SUR-L ( 6 )	0.13104494E+01	-7.23154328E-01	-1.08470149E+00
47.44013440E-01	F-SUR-0 ( 6 )	0.71160460E-01	5.51449672E-01	4.26323407E+02
17.00000000E+00	V-SUR-AU ( 6 )	-2.41044774E-01	1.60146367E+00	1.13447749E+03
17.00000000E+00	0.	1.43955224E-01	2.65146367E+00	1.14445450E+03
17.00000000E+00	0.	5.33955224E-01	3.70146367E+00	1.53011322E+03
17.00000000E+00	0.	4.43955224E-01	-2.41044774E+03	-7.23134324E+05
17.00000000E+00	0.	1.23395522E+00	1.54361940E+04	4.44444444E+04
07	Y-SUR-AM ( 6 )	0.	4.53061940E+04	1.36144444E+05
07	0.	0.	7.51361940E+04	2.25444444E+05
07	0.	0.	1.04446194E+05	3.14444444E+05
07	Y-SUR-AV ( 6 )	0.	4.50000000E+00	4.50000000E+00
07	0.	3.00000000E+00	3.00000000E+00	4.50000000E+00
07	V-SUR-A THETA ( 6 )	1.00000000E+00	1.00000000E+00	3.00000000E+00
07	0.	0.	0.	1.00000000E+00
07	V-SUR-A BETA ( 6 )	0.	0.	0.
07	0.	0.	0.	0.
07	Y-SUR-A PHI ( 6 )	0.	0.	1.00000000E+00
07	0.	0.	0.	0.

2.61963878E+01	P-SUR-L ( 7 )	7.85546817E+01	-8.43656716E+01	-1.47639925E+00
5.50196399E+01	F-SUR-G ( 7 )	7.85546817E+01	6.43683280E+01	8.26622530E+02
5.86526600E+01	F-SUR-G ( 7 )	9.01724292E+01	1.86884328E+00	1.13754516E+03
1.79000000E+00	Y-SUR-AU ( 7 )	-2.41944774E+01	3.09384328E+00	1.38629442E+03
1.70000000E+00	Y-SUR-AU ( 7 )	1.83955224E+01	4.31884328E+00	1.53211810E+03
1.70000000E+00	Y-SUR-AU ( 7 )	5.33955224E+01	-2.41044776E+05	-8.43656716E+05
1.70000000E+00	Y-SUR-AU ( 7 )	8.83955224E+01	1.56361940E+04	5.47266791E+04
1.70000000E+00	Y-SUR-AU ( 7 )	1.23395522E+00	4.51884328E+04	1.58851679E+05
0.0	Y-SUR-AU ( 7 )	0.0	7.51361940E+04	2.62876879E+05
0.0	Y-SUR-AU ( 7 )	0.0	1.04886194E+05	3.67101679E+05
0.0	Y-SUR-AU ( 7 )	0.0	6.12500000E+00	7.14583331E+00
0.0	Y-SUR-AU ( 7 )	3.50000000E+00	3.50000000E+00	6.12500000E+00
0.0	Y-SUR-AU ( 7 )	1.00000000E+00	1.00000000E+00	3.59000000E+00
0.0	Y-SUR-AU ( 7 )	0.0	0.0	1.00000000E+00
0.0	Y-SUR-AU ( 7 )	0.0	0.0	1.00000000E+00





201993027E-01	F-SUR=L ( % )	7.33190062E-01	-1.08470149E+00	-2.00057436E+00
0019071932E-01	F-SUR=D ( % )	9.66109474E-01	8.27798507E-01	8.27358353E+02
100000000E+00	V-SUR=AU ( % )	0.241044774E-01	2.40279851E+00	1.13968099E+03
100000000E+00		1.43955224E-01	3.97779851E+00	1.38983075E+03
100000000E+00		5.13955224E-01	5.55279851E+00	1.53705421E+03
100000000E+00		8.63955224E-01	-2.41044776E+05	-1.08470149E+06
100000000E+00		1.23394422E+00	1.56361940E+04	7.03620731E+04
00		0.	4.53861940E+04	2.00247473E+04
00		0.	7.51361940E+04	3.34112473E+05
00		0.	1.04886194E+05	4.71947473E+05
00		0.	1.01250000E+01	1.51875000E+01
00		4.50000000E+00	4.50000000E+00	1.01250000E+01
00		1.00000000E+00	1.00000000E+00	4.50000000E+00
00		0.	1.00000000E+00	4.50000000E+00
00		0.	0.	1.00000000E+00
00		0.	0.	0.



**APPENDIX D**  
**OUTPUT FOR ILLUSTRATIVE CASE A, B, C, AND D**  
**AND FOR THE TEST BEAM**





# CASE A

W	-0.78810E-02	-0.78840E-02	-1.92530E-03	3.89133E-03	1.91803E-01
VZ	0.	TIMEA	-0.55115E-17	BEYA	3.86229E-04
NY	-1.87027E-15	-0.11707E-15	1.62026E-14	-7.83135E-15	
VY	1.60000E-01	9.50000E-02	0.50000E-02	0.50000E-02	0.40000E-02
WY	3.21880E-01	1.70987E-01	1.70987E-01	1.70987E-01	1.70987E-01
Y1					
U	-0.68961E-08	-0.68961E-07	-1.11367E-08	2.25104E-08	1.10992E-06
W	-0.03333E-02	-0.21955E-02	-1.82951E-03	3.69023E-03	1.80608E-01
VZ	0.43059E-05	TIMEA	1.70000E-04	BEYA	2.95667E-04
NY	9.12551E-02	4.30415E-02	4.32020E-02	0.28544E-02	
VY	1.03711E-01	0.38188E-02	0.64990E-02	0.64910E-02	0.11130E-02
WY	2.66383E-01	1.30893E-01	1.30893E-01	1.30893E-01	1.30893E-01
Y1					
U	0.470011E-08	-1.06610E-06	-2.11601E-08	0.27861E-08	2.08907E-04
W	-7.45172E-02	-7.70289E-02	-1.55313E-03	3.13930E-03	1.50957E-01
VZ	1.62897E-04	TIMEA	2.97177E-04	BEYA	2.16173E-04
NY	3.71650E-02	7.51659E-02	7.50820E-02	7.64345E-02	
VY	1.31649E-01	0.29394E-02	0.77799E-02	0.76882E-02	7.82029E-02
WY	1.78277E-01	0.68163E-02	0.49163E-02	0.48163E-02	0.48163E-02
Y1					
U	-1.21394E-07	-1.25723E-06	-2.91122E-08	5.08467E-08	2.95963E-06
W	-5.19792E-02	-5.50296E-02	-1.12589E-03	2.27610E-03	1.07534E-01
VZ	3.159078E-04	TIMEA	3.85306E-04	BEYA	1.39241E-04
NY	0.82066E-02	0.74032E-02	0.83016E-02	0.64081E-02	
VY	1.23189E-01	0.23330E-02	0.86049E-02	0.84800E-02	7.61823E-02
WY	1.16034E-01	0.16833E-02	0.16433E-02	0.16433E-02	0.16433E-02
Y1					
U	-1.48061E-07	-1.70883E-06	-3.82023E-08	6.91573E-08	3.58097E-06
W	-0.06722E-02	-2.86030E-02	-5.90923E-04	1.19468E-03	5.66647E-02
VZ	9.82188E-04	TIMEA	4.37120E-04	BEYA	0.89712E-04
NY	3.47035E-02	1.10466E-01	1.11610E-01	1.09297E-01	
VY	1.16161E-01	0.19771E-02	0.90905E-02	0.89463E-02	7.00030E-02
WY	0.914310E-02	3.03573E-02	3.03573E-02	3.03573E-02	3.03573E-02

# CASE A

U	-1.401028E-07	-1.709043E-06	-3.595031E-08	7.267920E-08	3.506917E-06
M	-0.771472E-12	-2.100510E-13	-2.070330E-13	3.161915E-13	3.466132E-13
VZ	7.460250E-04	THEYA	2.502270E-04	BETA	1.566672E-17 PHI
QZ	9.600000E-02	1.147791E-01	1.159750E-01	1.135002E-01	
VUJ	1.169495E-01	0.105905E-02	0.023070E-02	0.009730E-02	7.460001E-02
WZ	1.305560E-14	0.035700E-15	0.035700E-15	0.035700E-15	0.035700E-15
U	-1.420010E-07	-1.700030E-06	-3.420230E-08	6.013732E-08	3.340070E-06
M	2.406722E-02	2.400301E-02	3.909250E-04	-1.196000E-03	-5.606607E-02
VZ	0.000022E-04	THEYA	4.571201E-04	BETA	-0.857172E-05 PHI
QZ	9.070335E-02	1.104007E-01	1.116100E-01	1.092970E-01	
VUJ	1.101010E-01	0.197710E-02	0.009050E-02	0.094030E-02	7.499300E-02
WZ	-0.710310E-02	-3.035727E-02	-3.035727E-02	-3.035727E-02	-3.035727E-02
U	-1.213001E-07	-1.057251E-06	-2.011225E-08	5.000070E-08	2.054001E-06
M	5.507021E-02	5.502005E-02	1.125000E-03	-2.276101E-03	-1.070500E-01
VZ	1.197720E-03	THEYA	3.053000E-04	BETA	-1.352010E-04 PHI
QZ	0.020002E-02	9.740320E-02	9.030102E-02	9.600102E-02	
VUJ	1.231001E-01	0.233300E-02	0.000000E-02	0.000000E-02	7.610230E-02
WZ	-1.100305E-01	-0.100333E-02	-0.100333E-02	-0.100333E-02	-0.100333E-02
U	-0.470011E-08	-1.006100E-06	-2.116001E-08	0.270013E-08	2.009070E-06
M	7.051720E-02	7.702005E-02	1.553135E-03	-3.139005E-03	-1.409007E-01
VZ	1.500050E-03	THEYA	2.071771E-04	BETA	-2.101735E-04 PHI
QZ	3.710005E-02	7.510500E-02	7.500205E-02	7.401005E-02	
VUJ	1.310007E-01	0.293000E-02	0.777900E-02	0.700022E-02	7.020200E-02
WZ	-1.900700E-01	-0.001000E-02	-0.001000E-02	-0.001000E-02	-0.001000E-02
U	-0.600010E-08	-0.600007E-07	-1.113075E-08	2.251000E-08	1.109002E-06
M	0.033333E-02	0.219557E-02	1.020517E-03	-3.400023E-03	-1.000000E-01
VZ	1.000055E-03	THEYA	1.700000E-04	BETA	-2.000000E-04 PHI
QZ	0.100113E-02	0.100113E-02	0.100113E-02	0.100113E-02	0.100113E-02

CASE A

VUJ	1.037111E-01	0.301000E-02	0.650000E-02	0.650103E-02	0.111302E-02
WUJ	-0.003030E-01	-0.300000E-01	-0.300000E-01	-0.300000E-01	-0.300000E-01
VC	10)				
U	1.030000E-17	2.775500E-17	3.000000E-18	-5.200170E-18	-1.307770E-17
W	0.000101E-02	0.700000E-02	1.025307E-03	-3.001531E-03	-0.010000E-01
VZ	1.930000E-03	0.	BETA	-3.002200E-00	PHI -1.020000E-00
WZ	-3.000000E-19	1.770000E-18	0.331701E-19	0.331701E-19	
VUJ	1.000000E-01	0.500000E-02	0.500000E-02	0.500000E-02	0.500000E-02
WUJ	-3.010000E-01	-1.700000E-01	-1.700000E-01	-1.700000E-01	-1.700000E-01
PT(00)	3.300000E+03				

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CASE A

M	00.00357E-01	-0.09711E-01	-7.90277E-02	2.10034E-01	1.672663E+00
VEE	-0.451115E+17	THETA	-0.163336E+17	BETA	5.354590E+04 PHI
QUJ	-0.076823E+13	-2.05671E+14	-3.355823E+13	1.373988E+14	
VUJ	1.400000E+01	8.500000E+02	8.500000E+02	8.500000E+02	8.500000E+02
WUJ	4.628825E-01	2.459063E-01	2.459063E-01	2.459063E-01	2.459063E-01
VE	1)				
U	-1.462524E-06	-1.392023E-05	-1.370042E-06	3.725642E-06	2.477079E-05
M	-0.307907E-01	-7.498692E-01	-7.467957E-02	2.037555E-01	1.551248E+00
VEE	5.298804E+04	THETA	6.2498113E+04	BETA	3.299032E+04 PHI
QUJ	0.237823E+02	1.568651E-01	1.623064E-01	1.477438E-01	
VUJ	1.004574E-01	8.127238E-02	9.102584E-02	8.988567E-02	6.959731E-02
WUJ	2.790943E-01	1.460977E-01	1.460977E-01	1.460977E-01	1.460977E-01
VE	2)				
U	-0.741618E-06	-2.499118E-05	-2.598000E-06	7.070531E-06	5.377283E-05
M	-7.424132E-01	-6.083605E-01	-6.318398E-02	1.722098E-01	1.261828E+00
VEE	1.417710E-03	THETA	1.033795E-03	BETA	1.429854E-04 PHI
QUJ	1.350828E-01	2.470575E-01	2.570745E-01	2.300418E-01	
VUJ	6.615810E-02	7.932439E-02	9.457356E-02	9.244659E-02	6.031203E-02
WUJ	1.608212E-01	8.503627E-02	8.503627E-02	8.503627E-02	8.503627E-02
VE	3)				
U	-3.720680E-06	-3.513893E-05	-3.563511E-06	9.704826E-06	7.274832E-05
M	-0.455124E-01	-4.232616E-01	-4.384358E-02	1.242404E-01	8.794777E-01
VEE	3.548956E+03	THETA	1.253797E-03	BETA	1.063091E-04 PHI
QUJ	1.647703E-01	2.972951E-01	3.106578E-01	2.743103E-01	
VUJ	8.701628E-02	7.835866E-02	9.659480E-02	9.372169E-02	5.886947E-02
WUJ	8.875758E-02	4.715247E-02	4.715247E-02	4.715247E-02	4.715247E-02
VE	4)				
U	-0.331640E-06	-0.060222E-05	-0.178251E-06	1.158899E-05	8.439146E-05
M	-7.752933E-01	-2.161712E-01	-2.372479E-02	6.499538E-02	4.501560E-01
VEE	4.926080E+03	THETA	1.566824E-03	BETA	4.730022E-05 PHI
QUJ	1.402722E-01	3.220891E-01	3.378605E-01	2.958157E-01	
VUJ	3.740602E-02	7.792849E-02	9.762716E-02	9.420248E-02	5.206041E-02
WUJ	1.941695E-02	2.094030E-02	2.094030E-02	2.094030E-02	2.094030E-02



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CASE A

U	-0.938040E-06	-0.271381E-05	-0.380059E-06	1.196655E-05	0.053705E-05
M	1.115552E-12	2.030809E-10	-2.131028E-14	2.664555E-14	1.021045E-13
VE	7.411232E-03	THEYA	1.001902E-03	BEYA	3.035766E-10 PMJ -3.028207E-05
QJ	1.850836E-01	3.301294E-01	3.461683E-01	3.022108E-01	
VJ	3.480868E-02	7.780810E-02	9.794351E-02	9.465369E-02	9.118265E-02
WJ	2.520005E-15	1.343950E-15	1.343950E-15	1.343950E-15	1.343950E-15
U	-0.331640E-06	-0.000222E-05	-0.478251E-06	1.138899E-05	0.055196E-05
M	2.752933E-01	2.161712E-01	2.372879E-02	-0.497938E-02	-0.501560E-01
VE	9.696335E-03	THEYA	1.366820E-03	BEYA	-0.730022E-05 PMJ -3.407086E-05
QJ	1.602728E-01	3.228891E-01	3.378605E-01	2.958157E-01	
VJ	3.740602E-02	7.792869E-02	9.762716E-02	9.429208E-02	9.206041E-02
WJ	-3.941685E-02	-2.094020E-02	-2.094020E-02	-2.094020E-02	-2.094020E-02
U	-5.720440E-06	-5.515893E-05	-5.563511E-06	9.704826E-06	7.274812E-05
M	9.359129E-01	9.232616E-01	9.588356E-02	-1.242402E-01	-0.799777E-01
VE	1.167291E-02	THEYA	1.293797E-03	BEYA	-1.065091E-08 PMJ -0.653331E-05
QJ	1.607703E-01	2.972951E-01	3.106578E-01	2.703103E-01	
VJ	4.701628E-02	7.835886E-02	9.659880E-02	9.372169E-02	9.488967E-02
WJ	-0.875736E-02	-0.715207E-02	-0.715207E-02	-0.715207E-02	-0.715207E-02
U	-2.791615E-06	-2.599116E-05	-2.598806E-06	7.070311E-06	9.377203E-05
M	7.689152E-01	6.083805E-01	6.318398E-02	-1.722098E-01	-1.261828E+00
VE	1.540474E-02	THEYA	1.033705E-03	BEYA	-1.029858E-08 PMJ -7.128126E-05
QJ	1.350824E-01	2.470575E-01	2.570765E-01	2.300818E-01	
VJ	6.615510E-02	7.938459E-02	9.457356E-02	9.246859E-02	6.031203E-02
WJ	-1.408212E-01	-0.543627E-02	-0.543627E-02	-0.543627E-02	-0.543627E-02
U	-1.409552E-06	-1.592023E-05	-1.370042E-06	3.725682E-06	2.877078E-05
M	0.502907E-01	7.488692E-01	7.487957E-02	-2.037555E-01	-1.551248E+00
VE	1.460302E-02	THEYA	6.008115E-08	BEYA	-3.249932E-08 PMJ -1.154021E-08
QJ	6.627935E-02	1.988891E-01	1.623069E-01	1.877818E-01	

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CASE A

WJ	1.004874E-01	0.127234E-02	9.102564E-02	0.988567E-02	0.99731E-02
WJ	-2.340003E-01	-1.460907E-01	-1.460907E-01	-1.460907E-01	-1.460907E-01
VF	10)				
U	2.818026E-18	2.081668E-17	1.738723E-17	1.778092E-17	0.673617E-18
W	0.009379E-01	0.097114E-01	7.902772E-02	-2.180342E-01	-1.672643E+00
VE	1.522206E-02	THETA	2.775598E-17	BETA	-5.58590E-08 PHI
WJ	-1.810878E-15	0.628266E-15	6.774963E-15	1.373988E-14	
WJ	1.600000E-01	0.500000E-02	0.500000E-02	0.500000E-02	0.500000E-02
WJ	-0.628832E-01	-2.459063E-01	-2.459063E-01	-2.459063E-01	-2.459063E-01
PI(EP)	0.237992E+03				

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9.797870E+0  
 2.35000E+0  
 1.91743E+0

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CASE B

U	-0.039160E-02	-0.000120E-02	2.135297E-02	1.760417E-01	2.270349E-06
VE	-1.736732E-17	7ME7A	6.071532E-18	BE7A	1.073308E-05 PHI -2.370710E-06
QU	1.950035E-16	6.240301E-15	7.400649E-15	6.0.050073E-10	
VU	2.633030E-03	2.075600E-01	2.475600E-01	2.633030E-03	1.032606E-12
WU	1.100566E-02	1.110112E+00	1.110112E+00	1.100566E-02	0.283060E-12
XC	1)				
U	-0.000670E-06	-1.037040E-06	6.102206E-09	1.076393E-07	0.052206E-07
W	-0.075000E-02	-0.010360E-02	2.010400E-02	1.667203E-01	1.040533E-04
VE	0.076730E-06	7ME7A	0.560508E-06	BE7A	0.020092E-06 PHI -2.240030E-06
QU	1.007080E-02	2.106616E-02	1.001753E-02	0.057403E-09	
VU	3.093973E-03	2.025007E-01	2.000304E-01	3.903200E-03	0.050217E-11
WU	0.003722E-03	0.775006E-01	0.775006E-01	0.303722E-03	0.901293E-12
XC	2)				
U	-1.679194E-07	-3.600300E-08	7.002000E-09	2.070803E-07	1.750417E-06
W	-7.000036E-02	-7.007000E-02	1.700000E-02	1.595606E-01	1.597040E-06
VE	1.030670E-05	7ME7A	1.000477E-05	BE7A	0.220123E-06 PHI -2.150072E-06
QU	1.017000E-02	3.022123E-02	3.019020E-02	0.900304E-09	
VU	3.000712E-03	2.300001E-01	2.025530E-01	0.200706E-03	0.000916E-11
WU	0.003500E-03	0.003001E-01	0.003001E-01	0.003500E-03	0.003007E-12
XC	3)				
U	-2.291055E-07	-5.013003E-08	1.000700E-08	0.070015E-07	2.310202E-06
W	-0.010700E-02	-0.012000E-02	1.220303E-02	0.007002E-02	0.303055E-09
VE	3.000750E-05	7ME7A	2.200300E-05	BE7A	0.103006E-06 PHI -2.007073E-06
QU	3.001070E-02	0.000910E-02	0.000910E-02	0.042071E-09	
VU	3.001660E-03	2.302130E-01	2.010030E-01	0.703006E-03	0.035307E-11
WU	0.003222E-03	0.270120E-01	0.270120E-01	0.003222E-03	0.100000E-12
XC	4)				
U	-2.000000E-07	-0.000000E-08	1.270200E-08	0.770000E-07	2.000200E-06
W	-2.002020E-02	-2.011017E-02	0.307011E-03	0.197577E-02	0.000020E-09
VE	0.177700E-05	7ME7A	2.911200E-05	BE7A	2.030333E-06 PHI -2.000110E-06
QU	2.000020E-02	5.002700E-02	5.002231E-02	0.000010E-09	
VU	0.003000E-03	2.300730E-01	2.002070E-01	0.003000E-03	0.070210E-11
WU	0.003000E-03	2.120200E-01	2.120200E-01	0.003000E-03	1.570000E-12

CASE B

U	-2.41673E-07	-6.168920E-08	1.34054E-08	5.00698E-07	2.76112E-06
W	-8.61092E-10	6.11066E-13	-2.01003E-12	1.527667E-13	2.66678E-21
VZ	8.75497E-08	1.461312E-05	8.75497E-08	-1.80735E-19	PMI -2.038030E-06
QXJ	2.96302E-02	5.91299E-02	5.265187E-02	4.701608E-09	
VZJ	1.60798E-03	2.36163E-01	2.399173E-01	5.152930E-03	5.09373E-11
WZJ	-2.10880E-16	-1.976610E-14	-1.976610E-14	-2.10880E-16	-1.66628E-25
U	-2.60809E-07	-5.66989E-09	1.27626E-08	4.77090E-07	2.649240E-06
W	2.92426E-02	2.911017E-02	-6.38701E-03	-5.19757E-02	-4.69662E-09
VZ	1.15516E-04	2.51126E-05	8.75497E-08	-2.03803E-06	PMI -2.08611E-06
QXJ	2.80824E-02	5.68274E-02	5.68223E-02	4.68818E-09	
VZJ	1.43430E-03	2.36676E-01	2.40207E-01	5.05929E-03	5.07921E-11
WZJ	-2.86199E-03	-2.12429E-01	-2.12429E-01	-2.26199E-03	-1.57188E-12
U	-2.39195E-07	-5.01309E-08	1.08870E-08	4.07601E-07	2.31424E-06
W	5.61079E-02	5.91240E-02	-1.22303E-02	-9.08798E-02	-9.36364E-09
VZ	1.371120E-04	2.20430E-05	8.75497E-08	-2.03803E-06	PMI -2.08773E-06
QXJ	2.90197E-02	4.88914E-02	4.88914E-02	4.64207E-09	
VZJ	1.69146E-03	2.362130E-01	2.410330E-01	4.76306E-03	5.03530E-11
WZJ	-4.55322E-03	-4.27612E-01	-4.27612E-01	-4.55322E-03	-3.16809E-12
U	-1.67919E-07	-3.66430E-08	7.98209E-09	2.97803E-07	1.75841E-06
W	7.80893E-02	7.80708E-02	-1.70006E-02	-1.39984E-01	-1.39704E-08
VZ	1.56792E-04	1.68847E-05	8.75497E-08	-2.03803E-06	PMI -2.15880E-06
QXJ	1.91795E-02	3.62213E-02	3.61582E-02	4.96630E-09	
VZJ	5.64871E-03	2.38808E-01	2.42553E-01	4.28874E-03	4.96016E-11
WZJ	-6.90359E-03	-6.80346E-01	-6.80346E-01	-6.90359E-03	-4.80346E-12
U	-4.86669E-08	-1.03794E-08	4.19228E-09	1.57639E-07	9.45224E-07
W	9.57500E-02	9.51936E-02	-2.01909E-02	-1.66729E-01	-1.68533E-08
VZ	1.90162E-04	2.56859E-05	8.75497E-08	-2.03803E-06	PMI -2.24893E-06
QXJ	1.88459E-02	3.16861E-02	1.04179E-02	4.87693E-09	



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CASE B

VRJ	1.003673E-03	2.428047E-01	2.446304E-01	3.503202E-03	0.454217E-11
VRJ	-0.343322E-03	-0.775046E-01	-0.775046E-01	-0.343322E-03	-0.401293E-12
VRJ	10)				
U	1.102622E-10	-1.130412E-10	-3.794700E-10	0.336000E-10	-0.073617E-10
U	0.030140E-02	0.000324E-02	-2.139207E-02	-1.740017E-01	-2.278340E-08
VRJ	1.751395E-04	THETA	2.602005E-10	0.073300E-05	PMI -2.378710E-06
VRJ	0.059074E-15	1.565916E-10	-1.774622E-10	7.267282E-17	
VRJ	2.633050E-03	2.073660E-01	2.073660E-01	2.633050E-03	1.032646E-12
VRJ	-1.100566E-02	-1.110112E-00	-1.110112E-00	-1.100566E-02	-0.283060E-12
PT(EPR)	2.370056E+03				



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CASE C

W	-2.71223E-02	-1.000120E-02	0.67257E-02	3.41443E-07	5.19377E-07
VEE	-1.110223E-16	THEYA	0.93804E-18	BETA	0.08089E-05 PHI -2.53576E-05
QUJ	2.95910E-15	-7.89357E-15	7.40111E-16	-3.44250E-16	
VUJ	2.446223E-02	0.731129E-01	2.42487E-03	1.22954E-10	1.22954E-10
WUJ	7.67160E-02	1.13600E-00	5.82210E-03	2.45220E-10	2.45220E-10
Xi 1)					
U	-1.26684E-08	-3.24924E-08	0.49264E-07	0.06984E-06	6.17731E-06
W	-2.95088E-02	-1.08089E-02	0.40811E-02	2.82084E-07	4.28073E-07
VEE	7.095613E-06	THEYA	2.72077E-05	BETA	0.81493E-05 PHI -2.40126E-05
QUJ	9.88332E-03	1.01680E-02	3.45821E-07	2.10152E-07	
VUJ	2.22059E-02	0.69927E-01	3.13539E-03	1.04923E-08	0.19201E-09
WUJ	0.66422E-02	9.02084E-01	0.42342E-03	2.34837E-10	2.34837E-10
Xi 2)					
U	-9.59623E-08	-6.14300E-08	0.49651E-07	7.27789E-06	1.10194E-05
W	-9.14007E-02	-1.54849E-02	3.60850E-02	2.16039E-07	3.22304E-07
VEE	2.42634E-05	THEYA	0.03807E-05	BETA	3.60333E-05 PHI -2.43926E-05
QUJ	1.06316E-02	1.79911E-02	3.32233E-07	2.12740E-07	
VUJ	2.60789E-02	0.67872E-01	3.60191E-03	1.10645E-08	0.24011E-09
WUJ	3.07802E-02	0.72649E-01	3.44760E-03	1.74815E-10	1.74815E-10
Xi 3)					
U	-9.27083E-08	-8.40564E-08	1.14277E-06	9.58655E-06	1.44902E-05
W	-1.93232E-02	-1.10884E-02	2.60704E-02	1.43580E-07	2.15461E-07
VEE	5.449873E-05	THEYA	0.33810E-05	BETA	2.39363E-05 PHI -2.40493E-05
QUJ	1.36680E-02	2.35322E-02	3.56882E-07	2.14518E-07	
VUJ	2.39612E-02	0.65720E-01	0.06400E-03	1.11038E-08	0.27313E-09
WUJ	2.30913E-02	0.66596E-01	2.28893E-03	1.16063E-10	1.16063E-10
Xi 4)					
U	-3.04073E-08	-9.04035E-08	1.36149E-06	1.09786E-05	1.65770E-05
W	-0.01309E-03	-5.77253E-03	1.37853E-02	7.20556E-06	1.07604E-07
VEE	0.46424E-05	THEYA	7.23661E-05	BETA	1.19386E-05 PHI -2.39167E-05
QUJ	1.45972E-02	2.46305E-02	3.99337E-07	2.15578E-07	
VUJ	2.98108E-02	0.64649E-01	0.29962E-03	1.12549E-06	0.29981E-09
WUJ	1.19170E-02	2.22744E-01	1.16163E-03	9.78881E-11	9.78881E-11

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CASE C

vi a) u -0.032977E-08 -1.033144E-07 1.420004E-06 1.140377E-05 1.727505E-05  
w 2.259326E-12 -1.565194E-13 -3.037570E-13 -2.422514E-19 -2.400277E-19  
vge 1.856001E-04 THEYA 7.552957E-05 BEYA -2.374000E-14 PHI -2.305760E-05  
quj 1.620022E-02 2.793761E-02 3.602190E-07 2.159311E-07  
vuj 2.306376E-02 4.643424E-01 4.376731E-03 1.127059E-08 4.294365E-09  
muj -2.40031E-18 -4.604291E-14 -2.402325E-16 -1.240555E-23 -1.240555E-23  
vi b) u -5.400732E-08 -0.400355E-08 1.361459E-06 1.007861E-05 1.657709E-05  
w 0.011090E-03 3.772303E-03 -1.370550E-02 -7.205564E-08 -1.074064E-07  
vge 1.629757E-04 THEYA 7.230610E-05 BEYA -1.103066E-05 PHI -2.391678E-05  
quj 1.559726E-02 2.603944E-02 3.593370E-07 2.155709E-07  
vuj 2.300108E-02 4.646000E-01 4.299422E-03 1.125496E-08 4.292010E-09  
muj -1.151708E-02 -2.227466E-01 -1.141635E-03 -5.788013E-11 -5.788013E-11  
vi c) u -3.370003E-08 -0.405640E-08 1.162775E-06 9.566555E-06 1.440027E-05  
w 1.430232E-02 3.108000E-02 -2.647044E-02 -1.435907E-07 -2.154610E-07  
vge 1.071575E-04 THEYA 4.330108E-05 BEYA -2.303657E-05 PHI -2.400073E-05  
quj 1.166007E-02 2.353224E-02 3.566027E-07 2.145109E-07  
vuj 2.306126E-02 4.637294E-01 4.049007E-03 1.118306E-08 4.273135E-09  
muj -2.309131E-02 -0.465946E-01 -2.208935E-03 -1.160035E-10 -1.160035E-10  
vi d) u -2.306236E-08 -0.143400E-08 0.406516E-07 7.277809E-06 1.101005E-05  
w 2.140007E-02 1.508003E-02 -3.604500E-02 -2.140397E-07 -3.223040E-07  
vge 2.255526E-04 THEYA 4.030076E-05 BEYA -3.605330E-05 PHI -2.400277E-05  
quj 1.005108E-02 1.709113E-02 3.522303E-07 2.127401E-07  
vuj 4.007007E-02 4.674722E-01 3.601910E-03 1.106499E-08 4.240110E-09  
muj -3.670025E-02 -0.726091E-01 -3.447605E-03 -1.746155E-10 -1.746155E-10  
vi e) u -1.266000E-08 -3.249200E-08 4.402000E-07 4.060000E-06 6.177317E-06  
w 2.550000E-02 1.548000E-02 -0.400113E-02 -2.028000E-07 -4.280730E-07  
vge 2.445500E-04 THEYA 2.729770E-05 BEYA -4.634950E-05 PHI -2.401200E-05  
quj 0.000000E-03 1.010000E-02 3.450210E-07 8.101500E-07

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CASE C

VRJ	2.020541E-02	4.000274E-01	3.135300E-03	1.000231E-00	0.102016E-00
VRJ	-0.666224E-02	-0.020004E-01	-0.623429E-03	-2.300372E-10	-2.300372E-10
VR	10)				
VR	1.127570E-17	-0.330094E-10	-2.001608E-17	1.000342E-17	-2.001608E-17
VR	2.712223E-02	1.000120E-02	-0.672297E-02	-3.014030E-07	-3.193770E-07
VR	2.916102E-04	THETA	-6.703000E-10	0.000600E-09	PMI -2.935707E-05
VR	3.020371E-19	5.702906E-15	-0.296455E-16	0.010202E-16	
VR	2.000023E-02	0.731129E-01	2.020027E-03	1.220950E-10	1.220950E-10
VR	-0.073602E-02	-1.130002E-00	-5.022309E-03	-2.032201E-10	-2.032201E-10
VR	2.000051E-04				





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CASE D

M	-1.22032E-01	-2.68119E-01	1.95991E-01	2.56159E-01	1.18561E-07
VE	0.	THEYA	1.73072E-18	BETA	9.83510E-05 PHI
QJ	-1.68373E-15	2.26809E-18	-2.49309E-18	1.83013E-16	-6.29913E-05
VJ	5.20029E-02	2.23099E-01	2.23099E-01	1.31690E-03	5.20029E-12
WJ	8.19992E-02	3.40326E-01	3.40326E-01	2.05619E-03	8.19992E-12
YI	1)				
U	-8.01528E-08	-7.71733E-07	5.66764E-07	3.17134E-06	5.67609E-06
M	-1.71021E-01	-2.51014E-01	1.80337E-01	2.36078E-01	9.05092E-08
VE	1.10619E-05	THEYA	8.19268E-05	BETA	7.03034E-05 PHI
QJ	8.17863E-02	1.04006E-01	6.04774E-02	6.24681E-08	-6.02023E-05
VJ	8.30913E-02	2.10906E-01	2.15379E-01	3.48775E-03	2.16540E-10
WJ	8.87329E-02	2.09696E-01	2.49696E-01	1.47399E-03	5.87528E-12
YI	2)				
U	-1.68070E-07	-1.43321E-06	1.06582E-06	3.96751E-06	9.89761E-06
M	-1.42262E-01	-2.08004E-01	1.53682E-01	1.46984E-01	7.68979E-08
VE	3.06679E-05	THEYA	7.14651E-05	BETA	4.64444E-05 PHI
QJ	7.14401E-02	1.77380E-01	1.01066E-01	4.75902E-08	-5.06520E-05
VJ	3.66194E-02	2.02690E-01	2.10092E-01	4.87166E-03	2.61253E-10
WJ	8.03704E-02	1.71574E-01	1.71574E-01	1.01281E-03	4.05706E-12
YI	3)				
U	-2.79932E-07	-1.08045E-06	1.45562E-06	8.12694E-06	1.31326E-05
M	-1.01227E-01	-1.47808E-01	1.09565E-01	1.39471E-01	5.22839E-08
VE	8.08671E-05	THEYA	9.10216E-05	BETA	3.03001E-05 PHI
QJ	9.11009E-02	2.25178E-01	1.26839E-01	5.07371E-08	-3.34196E-05
VJ	3.20022E-02	1.97453E-01	2.06365E-01	5.77227E-03	2.56470E-10
WJ	2.32901E-02	1.07312E-01	1.07312E-01	6.33474E-04	2.62401E-12
YI	4)				
U	-2.68013E-07	-2.31216E-06	1.70193E-06	9.46353E-06	1.51021E-05
M	-9.26756E-02	-7.64943E-02	5.68707E-02	7.20592E-02	2.63974E-06
VE	1.20001E-06	THEYA	1.02166E-06	BETA	1.45731E-05 PHI
QJ	1.02483E-01	2.22036E-01	1.41074E-01	5.24388E-08	-2.49007E-05
VJ	3.00039E-02	1.90551E-01	2.04197E-01	6.26838E-03	2.64476E-10
WJ	1.21440E-02	9.16140E-02	5.16140E-02	3.04688E-04	1.21440E-12

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CASE D

U	9.010101E-07	-2.029208E-06	1.700702E-06	9.99910E-06	1.575325E-05
M	9.700963E-13	-1.002140E-13	-5.32071E-14	-2.006900E-14	-5.509714E-21
VEE	1.017031E-04	THEYA	1.057067E-04	BETA	1.026303E-10 PHI -2.076053E-05
QUJ	1.001700E-01	2.007072E-01	1.050200E-01	5.297292E-08	
VUJ	2.020337E-02	1.036227E-01	2.030060E-01	6.026612E-03	2.672021E-10
WUJ	1.359253E-16	5.759020E-16	5.759020E-16	3.000050E-16	1.559253E-26
YI	6)				
U	-9.000133E-07	-2.312163E-06	1.701930E-06	9.003336E-06	1.510219E-05
M	5.207560E-02	7.049033E-02	-5.007070E-02	-7.209925E-02	-2.639702E-08
VEE	2.300707E-04	THEYA	1.021600E-04	BETA	-1.497330E-05 PHI -2.090070E-05
QUJ	1.020033E-01	2.020033E-01	1.010700E-01	5.203600E-08	
VUJ	5.000000E-02	1.000000E-01	2.001973E-01	6.200307E-03	2.000701E-10
WUJ	-1.210000E-02	-5.101000E-02	-5.101000E-02	-3.000000E-04	-1.210000E-12
YI	7)				
U	-2.293900E-07	-1.000000E-06	1.000000E-06	8.126000E-06	1.313200E-05
M	1.012270E-01	1.070000E-01	-1.000000E-01	-1.390710E-01	-5.220300E-08
VEE	2.020000E-04	THEYA	0.102100E-05	BETA	-3.030015E-05 PHI -3.301001E-05
QUJ	9.110001E-02	2.051700E-01	1.200300E-01	5.073710E-08	
VUJ	5.200022E-02	1.070000E-01	2.003000E-01	5.772270E-03	2.000700E-10
WUJ	-9.525010E-02	-1.073120E-01	-1.073120E-01	-6.350700E-04	-2.525010E-12
YI	8)				
U	-1.000700E-07	-1.033210E-06	1.000000E-06	5.007010E-06	9.007017E-06
M	1.022020E-01	2.000000E-01	-1.530023E-01	-1.000000E-01	-7.000000E-08
VEE	5.200000E-04	THEYA	7.100010E-05	BETA	-0.000000E-05 PHI -3.005200E-05
QUJ	7.100010E-02	1.775000E-01	1.010000E-01	4.759021E-08	
VUJ	5.001301E-02	2.000000E-01	2.100072E-01	0.071000E-03	2.012550E-10
WUJ	-0.037001E-02	-1.715700E-01	-1.715700E-01	-1.012013E-03	-0.037001E-12
YI	9)				
U	-0.013200E-08	-7.717303E-07	5.007000E-07	3.171300E-06	5.070000E-06
M	1.710210E-01	2.010000E-01	-1.000000E-01	-2.300701E-01	-0.000000E-08
VEE	5.000000E-04	THEYA	0.100010E-05	BETA	-7.000000E-05 PHI -0.020200E-05
QUJ	0.170000E-02	1.000000E-01	0.007000E-01	0.000000E-02	

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CASE D

VRJ	0.509131E-02	2.109066E-01	2.195703E-01	3.407757E-03	2.163000E-10
WRJ	-0.079285E-02	-2.406906E-01	-0.240690E-01	-1.473902E-03	-5.079285E-12
XC	10)				
U	2.001556E-10	-0.210000E-10	-0.743305E-10	2.002065E-10	-0.073617E-10
W	1.020331E-01	2.401195E-01	-1.000102E-01	-2.361590E-01	-1.105010E-07
VGE	3.439002E-00	THETA	0.	BEYA	-0.035100E-05
VRJ	0.063030E-15	1.132007E-10	-3.232007E-10	3.009007E-10	
VRJ	5.200295E-02	2.230951E-01	2.230951E-01	1.316003E-03	5.200295E-12
WRJ	-0.109020E-02	-3.403200E-01	-3.403200E-01	-2.056100E-03	-0.109020E-12
PS(EAP)	1.432500E+00				

**TEST BEAM (Reference 1)**

[illegible][illegible]



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TEST BEAM

2.240313E+01  
1.020446E+00

VI 01  
" 1.461251E+17 4.934800E-18 -9.673617E-19 4.336800E-14  
" 1.500440E+05 -1.230302E+00 1.230303E+00 1.000407E-06 2.073345E-06  
" 2.774455E+13 THEVA 1.387770E-17 BETA 1.000204E-08 PMI -2.466210E-05  
" 1.470272E+15 1.050411E-14 2.491247E-16 -1.063464E-16  
" 2.472060E+09 4.974500E-01 2.540546E-03 1.292791E-10 1.292791E-10  
" 1.431213E+08 3.541644E+00 1.415106E-02 9.204200E-10 9.204200E-10

ADJUDGMENT TON SHALL  
BEING NUMBER 115 DETECTED BY FIB

VI 11  
" 2.210440E-08 -9.003400E-06 4.111853E-05 7.000204E-05 1.027213E-04  
" 1.172240E+05 -1.190237E+00 1.190237E+00 1.633356E-06 2.073040E-06  
" 2.431597E+08 THEVA 2.007155E-08 BETA 1.440900E-08 PMI -2.900270E-05  
" 1.440212E+06 4.200017E-02 4.031240E-07 2.574533E-07  
" 1.410142E+07 4.817040E-01 4.002642E-03 1.340610E-08 5.100000E-08  
" 1.409110E+08 2.725000E+00 1.390000E-02 7.003620E-10 7.003620E-10

ADJUDGMENT TON SHALL  
BEING NUMBER 115 OFFERED BY FIB

VI 21  
" 4.000000E+08 -1.336303E+03 7.791077E-05 1.459077E-04 1.407440E-04  
" 4.000000E+08 -9.459270E+01 4.659440E-01 1.200970E-06 1.404911E-06  
" 4.000000E+08 THEVA 4.002642E-08 BETA 1.000001E-08 PMI -2.196330E-05  
" 1.750372E+06 1.300000E-01 9.051043E-07 2.855201E-07  
" 4.001122E+08 4.706104E-01 1.250235E-02 1.502501E-08 5.634500E-08  
" 1.024000E+08 1.000204E+00 1.017500E-02 9.190010E-10 9.190010E-10

VI 31  
" 4.001170E+04 -1.023122E+04 1.057410E-04 1.037402E-04 2.050473E-04  
" 4.002642E+06 -4.050000E+01 4.051020E-01 8.790000E-07 1.007797E-06  
" 1.000033E+03 THEVA 4.015070E-08 BETA 6.000170E-04 PMI -2.007410E-05  
" 1.000000E+08 1.001210E+01 4.000010E-07 9.000030E-07

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TEST BEAM

VUJ	A.030000E-00	0.032000E-01	1.033000E-02	1.037000E-00	1.000000E-00
VUJ	A.030000E-00	1.000000E-00	0.000000E-03	3.000000E-10	3.000000E-10
VI	A)				
U	-0.000000E-00	-0.120000E-00	1.230000E-00	2.035537E-00	2.017702E-00
U	-0.000000E-00	-0.000000E-01	3.000000E-01	0.000000E-07	5.000000E-07
VPE	1.030000E-03	1.030000E-03	1.030000E-03	1.030000E-03	-1.000000E-03
VUJ	1.030000E-00	1.000000E-01	5.000000E-07	3.000000E-07	
VUJ	0.000000E-00	0.000000E-01	1.000000E-02	1.000000E-00	0.000000E-00
VUJ	0.000000E-00	0.000000E-01	1.000000E-03	1.000000E-10	1.000000E-10
VI	A)				
U	-0.000000E-00	-0.000000E-00	1.000000E-00	2.035537E-00	2.001111E-00
U	0.000000E-00	0.000000E-01	-0.000000E-01	-0.000000E-01	-0.000000E-01
VPE	0.000000E-03	1.030000E-03	1.030000E-03	1.030000E-03	-1.000000E-03
VUJ	1.000000E-00	1.000000E-01	5.000000E-07	3.000000E-07	
VUJ	0.000000E-00	0.000000E-01	1.000000E-02	1.000000E-00	0.000000E-00
VUJ	-1.000000E-02	-0.000000E-01	-1.000000E-01	-0.000000E-01	-0.000000E-01
VI	A)				
U	-0.000000E-00	-0.120000E-00	1.230000E-00	2.035537E-00	2.017702E-00
U	0.000000E-00	0.000000E-01	-0.000000E-01	-0.000000E-01	-0.000000E-01
VPE	0.000000E-03	1.030000E-03	1.030000E-03	1.030000E-03	-1.000000E-03
VUJ	1.000000E-00	1.000000E-01	5.000000E-07	3.000000E-07	
VUJ	0.000000E-00	0.000000E-01	1.000000E-02	1.000000E-00	0.000000E-00
VUJ	-1.000000E-02	-0.000000E-01	-1.000000E-01	-0.000000E-01	-0.000000E-01
VI	A)				
U	-0.000000E-00	-0.120000E-00	1.030000E-00	1.030000E-00	2.000000E-00
U	0.000000E-00	0.000000E-01	-0.000000E-01	-0.000000E-01	-0.000000E-01
VPE	0.000000E-03	1.030000E-03	1.030000E-03	1.030000E-03	-1.000000E-03
VUJ	1.000000E-00	1.000000E-01	5.000000E-07	3.000000E-07	
VUJ	0.000000E-00	0.000000E-01	1.000000E-02	1.000000E-00	0.000000E-00
VUJ	-1.000000E-02	-0.000000E-01	-1.000000E-01	-0.000000E-01	-0.000000E-01
VI	A)				
U	-0.000000E-00	-0.120000E-00	1.030000E-00	1.030000E-00	2.000000E-00
U	0.000000E-00	0.000000E-01	-0.000000E-01	-0.000000E-01	-0.000000E-01
VPE	0.000000E-03	1.030000E-03	1.030000E-03	1.030000E-03	-1.000000E-03
VUJ	1.000000E-00	1.000000E-01	5.000000E-07	3.000000E-07	
VUJ	0.000000E-00	0.000000E-01	1.000000E-02	1.000000E-00	0.000000E-00
VUJ	-1.000000E-02	-0.000000E-01	-1.000000E-01	-0.000000E-01	-0.000000E-01
VI	A)				
U	-0.000000E-00	-0.120000E-00	1.030000E-00	1.030000E-00	2.000000E-00
U	0.000000E-00	0.000000E-01	-0.000000E-01	-0.000000E-01	-0.000000E-01
VPE	0.000000E-03	1.030000E-03	1.030000E-03	1.030000E-03	-1.000000E-03
VUJ	1.000000E-00	1.000000E-01	5.000000E-07	3.000000E-07	
VUJ	0.000000E-00	0.000000E-01	1.000000E-02	1.000000E-00	0.000000E-00
VUJ	-1.000000E-02	-0.000000E-01	-1.000000E-01	-0.000000E-01	-0.000000E-01

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